

Information Request Response

| То | The BUSY School C/- Bespoke P&D | Date | 16 June 2025 |
|----------------|---------------------------------------|-------------|--|
| Prepared by | Casey Schackow, Velocity, Director | Approved by | Harj Singh, Traffic Engineering Advisor (RPEQ 22364) |
| Location | 153 Dalrymple Road, Garbu | utt | |
| Status | Final | Attachments | Appendix A: Information Request Plans Appendix B: Development Plans Appendix C: Swept Path Assessment Appendix D: SIDRA Assessment |

Introduction

1.1 Overview

Velocity has been commissioned by The BUSY School C/- Bespoke P&D to provide traffic and transport advice in response to the items within the Information Request (IR) #MCU25/0013, for the proposed development located at 153 Dalrymple Road, Garbutt.

The issues have been extracted from the IR and replicated below, responses to each item have been summarised in this Information Request Response.



Item 1 – Confirmation of On-Site Parking Requirements

The applicant is requested to provide:

- a) Confirmation of the age of students attending the proposed educational establishment
- b) The number of students able to be collected via shuttle bus;
- c) Details of bus pick-up and set-down for the shuttle bus.

Item 2 – Traffic Impact Assessment

- a) The applicant is requested to provide an amended Traffic Impact Assessment (TIA) report certified by an RPEQ and which meets the requirements of a Moderate TIA in accordance with SC6.4 Development manual planning scheme policy of the Townsville City Plan. Per SC6.4.5.2(2)(c), "a moderate level TIA Report will be performed using traffic forecasting processes or computerised methods (e.g., Sidra) to estimate and analyse traffic flows on the surrounding road network at predefined time horizons."
- b) The amended TIA is also requested to include swept path analysis diagrams for the shuttle bus manoeuvres demonstrating that the shuttle bus can safely manoeuvre on the site and enter and exit in a forward gear



Design Amendments

The development plans have been updated to improve the safety and overall amenity for the future students at the site. The changes are focused on improving connectivity and capacity of active transport facilities to increase uptake.:

- The bicycle parking spaces have been relocated in front of the car parking bays with adequate space left for the bicycles to be parked in accordance with AS2890.3. Additional bicycle spaces have been provided, the new total is 16 spaces
- A scooter/skateboard parking area has been provided adjacent the main building entrance
- A footpath has been provided to link the bicycle parking and aforementioned scooter/skateboard parking to the front entrance, separated from all vehicle manoeuvres.
- The PWD space and shared space has been shifted back 1m to allow direct access from the new footpath
- A zebra crossing (pedestrian priority) has been provided across the main entrance crossover to ensure safety of pedestrians crossing into the site from Dalrymple Service Road.
 - A section of path has been added opposite the new zebra crossing as a landing for pedestrians which is then funnelled through a new footpath along the frontage to the west, where majority of active and public transport users will travel from.

These design updates are provided at Appendix B.



Velocity Response – Item 1a

Information has been sourced from the client regarding the ages of the student attending the proposed educational establishment. The ages are expected to be similar to those of the existing TBS facilities (and as such have been based on the existing attendance of the Ipswich and Cleveland sites.

The student profile has been determined by averaging the two surveyed sites. This is detailed in Table 1.

Table 1 Student Age Groups

| Facility | Years 11 (15-16 years old) | Years 12 (17-18 years old) | |
|----------------------------|----------------------------|----------------------------|--|
| Ipswich and Cleveland | 130 Students 120 Students | | |
| Garbutt (Average of above) | 65 Students | 60 Students | |

Velocity Response – Item 1b&c

A 22-seater Toyota Coaster (7m long) or similar shuttle bus is proposed to transport students to the proposed facility from two locations within the Townsville CBD.

The service will run twice in both the AM and PM periods, for a total of 4 times throughout the day. As such, a total of 44 students will be collected and brought to the campus each morning and returned to either of the Townsville CBD locations after school. This represents ~34% of the total quantum of students travelling to the educational establishment each day.

The Toyota Coaster is proposed to set-down within the site, in the available space adjacent the PWD parking space. This area has sufficient room to cater for the 7m long x 2.1m wide bus with adequate room for alighting. Specifically, the available area for the bus set down is proposed to measure 8m x 4m as shown indicatively in Figure 1.

Figure 1 Indicative Bus Set Down Area



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Parking Compliance

The parking requirements have been updated based on the rates specified for a Secondary Education land use and are shown in Table 1.

Table 1 Car Parking Requirements

| Land use | Yield | Car Parking Rate | Car Parking Required |
|------------------------|--|---|---|
| Secondary Education | 128 students (60 of which are of driving age) + 30 staff | 1 per FTE + 1 per 10 students of driving age + 1 visitor per 50 students | 30 FTE spaces + 6 student spaces + 3 visitors |

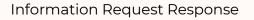
As shown in Table 1, the proposed development requires a total of 30 FTE spaces, 6 student spaces and 3 visitor spaces for a total of 39 parking spaces.

This is in contrast to the requirements of a Tertiary Education land use which requires 15 FTE spaces and 13 student spaces.

As such, there is a shortfall of 19 parking spaces.

However, these requirements do not take into consideration the following factors:

- Council has provided the previous tenant with 25 additional on-street parking spaces directly adjacent the facility.
- The 44 students that will be taken to and from the campus via the proposed shuttle bus service each day
- The various mode share options available to staff members including public transport (which are located nearby and are serviced frequently as detailed within the Velocity Traffic - Traffic Impact Assessment Report [VT TIA]), active transport and carpooling.
 - The provision of 30 staff parking spaces is considered excessive considering only 15 are required for a tertiary education facility which are similar in nature (considering the age of the students).
- The existing TBS sites at Cleveland and Shailer Park only have 20 students (data attainted from the client) and 6 students (as detailed in the VT TIA), respectively, that have their licence and drive to the campus
 - This is considerably lower than the 60 students (1000% more than Shailer Park) required to be used within the parking assessment
- Data from the Shailer Park Site (as detailed in the VT TIA) specified that 50% of the students utilised public transport to travel to the site. Adopting this rate

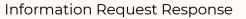




would see 64 students utilising the bus to attend the proposed site and significantly reduce short term parking demands.

- Visitors are not expected, nor are do they typically attend the existing TBS sites.
- The parking spaces currently provided at the existing TBS sites are as follows:
 - Cleveland (Redlands): 28 spaces
 - Shailer Park (Logan): 22 spaces
 - Strathpine (Moreton Bay): 22 spaces
 - Ipswich (Ipswich): 14 spaces
 - Southport (Gold Coast): 15 spaces
 - Coolangatta (Gold Coast): 25 spaces
 - All of these sites were approved for considerably less than the 39 spaces required by Townsville City Council and span across 5 different LGAs.

With respect to the above, there is expected to be far less than 39 parking spaces utilised on a daily basis to cater for the needs of the proposed TBS site. Velocity is of the opinion that the detail outlined within the VT TIA and the above, with particular focus on the 44 students utilising the shuttle bus and the additional 25 parking spaces on-street, that the proposed parking arrangements are considered adequate to service the expected demands.





Velocity Response – Item 2a

Velocity has undertaken a detailed intersection analysis using SIDRA Intersection software in accordance with the DTMR Guide to Traffic Impact Assessment (GTIA). This is in addition to the previously submitted Traffic Impact Assessment, which provided a high-level assessment based on trip generation and distribution.

Traffic volumes used for this analysis were sourced from the Townsville City Council's publicly available traffic calibration dataset. While this dataset is suitable for modelling purposes, it provides only generalised AM and PM peak period volumes and does not specify the exact peak hour, however, generally the AM road peak is located between 6-9am and the PM peak between 4-7pm.

Accordingly, we have taken a conservative approach by assuming the proposed school's PM peak period coincides with the broader network peak, despite the fact that the development's actual traffic peaks (2:45–3:45pm) are likely to occur outside the true background peak hours. This peak overlap assumption results in a highly conservative basis for modelling development impacts.

Furthermore, while our original TIA included the removal of church traffic for net impact estimates, we note that the church was operational at the time of background traffic data collection, and its traffic is therefore already embedded within the base volumes. To ensure a conservative approach, we have **not subtracted the church traffic from background volumes** in the SIDRA analysis. This represents an unrealistically worst-case scenario, modelling the new school traffic on top of the full background volumes, inclusive of the existing use.

Nonetheless, we have undertaken SIDRA analysis for all relevant intersections under both background and background with development scenarios as detailed in the following sections.

Development Traffic and Intersection Assessment

Proposed Shuttle Bus

A 22-seater privately contracted Toyota Coaster (7m long) or similar shuttle bus is proposed to transport students to the proposed facility from two locations within the Townsville CBD.

The service will run twice in the AM and PM periods, for a total of four times throughout the day. As such, 44 students will be collected and brought to the campus each morning and returned to either of the Townsville CBD locations after school. This represents ~34% of the total number of students travelling to the educational establishment each day.



Updated Traffic Generation

The traffic generation expected at the site has been updated based on the information detailed above. The updated traffic generation of the existing and proposed land uses is detailed within the following section.

The number of students travelling by their own vehicle has been assumed to be 20, in line with the number of students driving at the Cleveland TBS.

The modal splits from the VT TIA have been updated and detailed in Table 1, Table 2 and Table 3. The 44 shuttle bus users and 20 student drivers have been assumed to be evenly split between the three location ranges.

Table 1 Users Expected to Reside Within 2km of School

| Student located: 1-2km | | 30% | 38 Students | | ents | | |
|--------------------------|------------------|------------------------|---------------------------|------|------------------------|--|--------------|
| Mode | Private (Driver) | Private (Passenger) | Shuttle Bus | | Shuttle Bus Cycle + Wa | | Cycle + Walk |
| % of Student | 15.6% | 34.0% | 34.4% | 9.0% | 6.0% | | |
| # Student | 5.9 | 12.9 | 13 (13.1) 3 (3.4) 2 (2.3) | | 2 (2.3) | | |
| Occupancy rate of 1.2 | 5 (4.9) vehicles | 11 (10.8) vehicles | | | | | |

Table 2 Users Expected to Reside Within 2-5km of School

| Student located: 2-5km | | 50% | 64 Students | | ents |
|--------------------------|------------------|------------------------|--------------------|------|--------------|
| Mode | Private (Driver) | Private (Passenger) | Shuttle Bus Cycle | | Cycle + Walk |
| % of Student | 15.6% | 34.0% | 34.4% | 9.0% | 6.0% |
| # Student | 10 | 21.8 | 22 6 (5.8) 4 (3.8) | | 4 (3.8) |
| Occupancy rate of 1.2 | 8 (8.3) vehicles | 18 (18.1) vehicles | ; | | |

Table 3 Users Expected to Reside Within >5km of School

| Student l | ocated: >5km | 20% | 26 Students | | ents | |
|--------------------------|------------------|------------------------|-------------------------|-------|--------------|--|
| Mode | Private (Driver) | Private (Passenger) | Shuttle Bus | | Cycle + Walk | |
| % of Student | 15.6% | 30.5% | 34.4% | 17.0% | 1.5% | |
| # Student | 4.1 | 7.9 | 9 (8.9) 4 (4.4) 0 (0.4) | | 0 (0.4) | |
| Occupancy rate of 1.2 | 3 (3.3) vehicles | 7 (6.6) vehicles | | | | |

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Internal Trip Distribution

The updated existing and proposed traffic generation and directional input is detailed Table 4.

Table 4 Directional Trips

| | Yield | AM | | РМ | |
|---------------------|-----------------------------|--------|--------|--------|--------|
| Land Use | | IN | OUT | IN | OUT |
| | Student (Private) – 16 veh | 16 vph | 0 vph | 0 vph | 16 vph |
| The BUSY School | Student (Drop-off) – 36 pax | 36 vph | 36 vph | 36 vph | 36 vph |
| | Staff – 30 veh | 30 vph | 0 vph | 0 vph | 30 vph |
| NET PEAK TRAFFIC | | 63 vph | 17 vph | 17 vph | 63 vph |

As outlined in Table 4, an estimated net increase of 63 vph and 17 vph are expected to enter and exit the site, respectively, during the AM peak period, the opposite is true of the PM peak.

Study Intersections

Figure 1 shows the intersections under investigation, detail is provided in Table 5. No other intersections are expected to have any movement impacted enough to cause a net increase in delay of more than 5%.



Figure 1 Study Intersections

Source: Nearmap

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Table 5 Study Intersections

| ID | Intersection | Control |
|----|---|-------------------------|
| 1 | Dalrymple Road / Bayswater Road | Signalised Intersection |
| 2 | Bayswater Road / Pilkington Street | Roundabout |
| 3 | Bayswater Road / Dalrymple Service Road | Priority Intersection |
| 4 | Dalrymple Service Road / Site Access | Priority Intersection |

Background Traffic Volumes

As detailed, to understand the existing traffic conditions, road segment traffic counts were attained using the TCC Traffic Calibration Model 2025.

Assumptions have been adopted to estimate the directional flows at the roundabout of Bayswater Road / Pilkington Street and the signalised intersection of Dalrymple Road / Bayswater Road as the road segment volumes do not provide individual directional volumes. These assumptions have ensured that the arrival flows balance with the departure flow to ensure the network is in equilibrium.

Background Traffic Growth

To understand traffic growth within the transport network near the development site, TMR's 2023 Segment Reports were consulted. These reports provide information on the Average Annual Daily Traffic (AADT) on State Controlled Roads in Queensland.

AADT data from several nearby locations were used to provide insight into traffic near the development site. The 10-year growth rates were averaged to determine a conservative growth rate for the development. Based on the assessment, a growth rate of 1% was adopted.

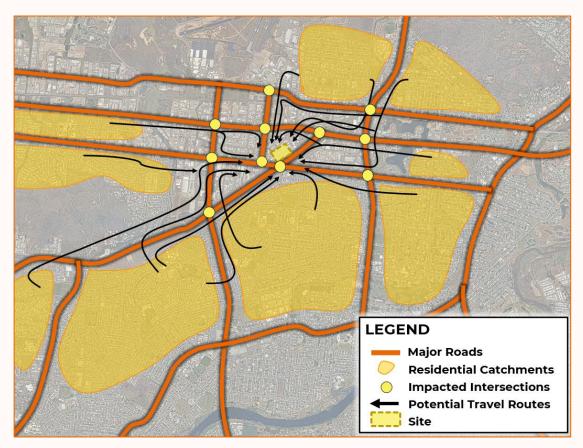
Trip Distribution

External Directional Distribution

The external trip distribution has been based on an expansion of the desktop model created within the VT TIA which is reproduced in Figure 2.



Figure 2 Expected Travel Routes to TBS



A more detailed assessment has been used to determine the travel routes and subsequent impacted intersections as a result of the proposed development.

The land area of each of the residential catchments detailed in Figure 2 was attained and a Google Maps travel route assessment was undertaken to determine the route chosen from each catchment area to the site during the typical AM peak. The chosen travel route was factored by the residential land area proportion.

The final external ingress and egress movement distribution are illustrated in Figure 3 and Figure 4.

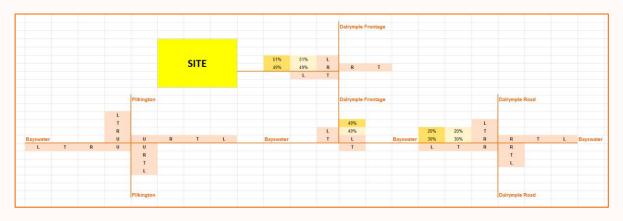


Figure 3 IN Distribution

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Figure 4 OUT Distribution



Assessment Scenarios

To evaluate the impact of the proposed development on the existing road network, each intersection has been analysed during the AM and PM peak periods, using the proposed yields detailed in Table 4.

As required by TMR's GTIA, the impact assessment year for external intersections should be the year of opening, while the impact assessment year for site accesses should be 10 years after the year of opening.

SIDRA results and layouts can be found in **Appendix D**. Table 6 summarises the impact assessment scenarios.

| Impact Assessment Scenario | Study Intersections |
|----------------------------------|---------------------|
| 2025 BG (Year of opening) | All |
| 2035 BG (10-year design horizon) | 4 |
| 2025 BG + DEV | All |
| 2035 BG + DEV | 4 |

Table 6 Assessment Scenarios

Note: BG = Background, Dev = Development Traffic

Assessment Criteria

Intersection Degree of Saturation

The performance of each study intersection has been analysed using SIDRA Intersection 9.0 to determine the Degree of Saturation (DOS), queuing and delays. DOS is a measure of the intersection's capacity. TMR's DOS thresholds are listed in Table 7



Table 7 Intersection DOS Thresholds

| Intersection Treatment | DOS Threshold |
|--------------------------|----------------------------|
| Signalised Intersections | Less than or equal to 0.90 |
| Priority Intersections | Less than or equal to 0.80 |
| Roundabouts | Less than or equal to 0.85 |

Source: TMR Guidelines for Assessment of Road Impacts Development

If the DOS exceeds the values in Table 7, it means that the intersection is approaching its practical capacity and upgrade works may be necessary. Above these threshold values, users of the intersection are likely to experience increased delays and queueing.

Intersection Delay

According to the *GTIA*, delay is a more important indicator of intersection performance than the DOS, as outlined in the previous TMR Guidelines for Assessment of Road Impacts of Development (GARID).

The GTIA aims to prevent intersection delays on the base traffic from getting worse by more than 5% on average as a result of the development. Measures to avoid, manage, or mitigate intersection delays must be considered if delays are expected to exceed this threshold. Velocity Traffic has provided additional comments if this threshold has been exceeded.

For priority-controlled intersections, if the average peak hour delay for any movement exceeds 42 seconds, as stated in the GTIA, the intersection should be upgraded for safety reasons. For signalised intersections, the DOS is still considered, as delay depends on the cycle length and phasing arrangement.

95th Percentile Queuing

The 95th percentile queue length is the one that 95% of all observed queue lengths during the assessment hour fall under. This value indicates the maximum queue length that should be designed for to prevent negative impacts on upstream lanes.

Modelling Parameters

A summary of the SIDRA parameters adopted for the study intersection is summarised below:

Signalised:

- Peak Flow Factor 0.95 (30min/60min)
- Basic Saturation Flow 1,950tcu/hr

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Intersection 1: Dalrymple Road / Bayswater Road

The Dalrymple Road / Bayswater Road intersection is a four-way signalised intersection as shown in Figure 5. Results from the SIDRA analysis are summarised in Table 8 and Table 9.

Full SIDRA results for this assessment can be found in **Appendix D.**

Figure 5 Dalrymple Road / Bayswater Road intersection - SIDRA Layout

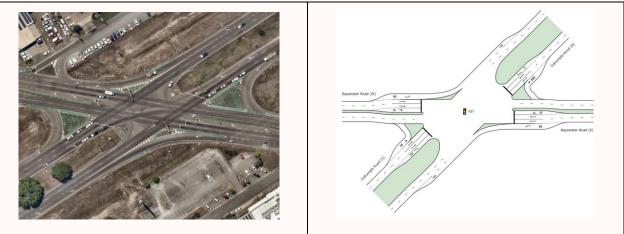


Table 8 Dalrymple Road / Bayswater Road intersection - SIDRA Summary (AM)

| Scenario | DOS | Critical Delay | 95th %ile Queue | Critical Movement |
|---------------|-------|----------------|-----------------|-------------------|
| 2025 BG | 0.879 | 101.1 sec | 288.3m | Western Right |
| 2025 BG + DEV | 0.892 | 104.3 sec | 292.1m | Western Right |

Table 9 Dalrymple Road / Bayswater Road intersection - SIDRA Summary (PM)

| Scenario | DOS | Critical Delay | 95th %ile Queue | Critical Movement |
|---------------|-------|----------------|-----------------|-------------------|
| 2025 BG | 0.880 | 63.4 sec | 202.4m | Southern Right |
| 2025 BG + DEV | 0.888 | 63.9 sec | 210.2m | Southern Right |

The results presented in Table 8 and Table 9 above suggest that the intersection is within acceptable performance thresholds for a signalised intersection (i.e. DOS less than 0.9) in all scenarios.

Additionally, it is clear that the addition of the proposed site traffic is negligible compared to the existing traffic and thus the performance of the signalised intersection.



Intersection 2: Bayswater Road / Pilkington Street

The Bayswater Road / Pilkington Street intersection is a four-way roundabout as shown in Figure 5. Results from the SIDRA analysis are summarised in Table 8 and Table 9.

Full SIDRA results for this assessment can be found in **Appendix D.**

Figure 6 Bayswater Road / Pilkington Street intersection - SIDRA Layout



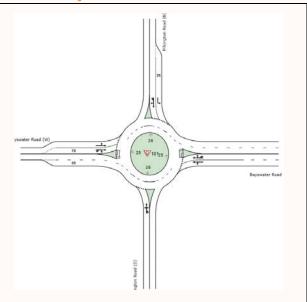


Table 10 Bayswater Road / Pilkington Street intersection - SIDRA Summary (AM)

| Scenario | DOS | Critical Delay | 95th %ile Queue | Critical Movement |
|---------------|-------|----------------|-----------------|-------------------|
| 2025 BG | 0.413 | 15.1 sec | 19.6m | Southern U-turn |
| 2025 BG + DEV | 0.421 | 15.2 sec | 20.2m | Southern U-turn |

Table 11 Bayswater Road / Pilkington Street intersection - SIDRA Summary (PM)

| Scenario | DOS | Critical Delay | 95th %ile Queue | Critical Movement |
|---------------|-------|----------------|-----------------|-------------------|
| 2025 BG | 0.287 | 13.8 sec | 11.9m | Southern U-turn |
| 2025 BG + DEV | 0.289 | 13.8 sec | 12m | Southern U-turn |

The results presented in Table 8 and Table 9 above suggest that the intersection is within acceptable performance thresholds for a roundabout (i.e. DOS less than 0.85) in all scenarios.



Intersection 3: Bayswater Road / Dalrymple Service Road

The Bayswater Road / Dalrymple Service Road intersection is a three-way prioritycontrolled intersection as shown in Figure 5. Results from the SIDRA analysis are summarised in Table 8 and Table 9.

Full SIDRA results for this assessment can be found in **Appendix D.**

Figure 7 Bayswater Road / Dalrymple Service Road intersection - SIDRA Layout

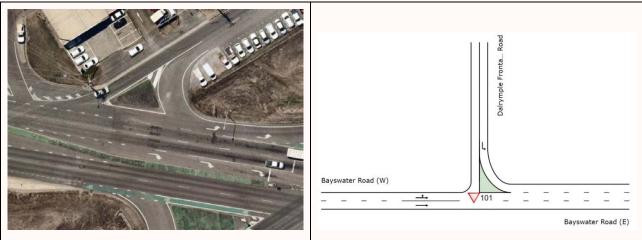


Table 12 Bayswater Road / Dalrymple Service Road intersection - SIDRA Summary (AM)

| Scenario | DOS | Critical Delay | 95th %ile Queue | Critical Movement |
|---------------|-------|----------------|-----------------|-------------------|
| 2025 BG | 0.230 | 5.7 sec | 0m | Western Left |
| 2025 BG + DEV | 0.244 | 5.7 sec | 0m | Western Left |

| Table 13 Bayswater Road / Dalrymple Service Road intersection - SIDRA Summary (PM) | | | | | | |
|--|--|---------|----|--------------|--|--|
| Scenario | DOS Critical Delay 95th %ile Queue Critical Moveme | | | | | |
| 2025 BG | 0.181 | 5.7 sec | 0m | Western Left | | |
| 2025 BG + DEV | 0.185 | 5.7 sec | 0m | Western Left | | |

The results presented in Table 8 and Table 9 above suggest that the intersection is within acceptable performance thresholds for a priority-controlled intersection (i.e. DOS less than 0.8) in all scenarios.



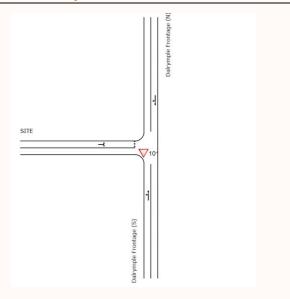
Intersection 4: Dalrymple Service Road / Site Access

The Dalrymple Service Road / Site Access intersection is a three-way priority-controlled intersection as shown in Figure 5. Results from the SIDRA analysis are summarised in Table 8 and Table 9.

Full SIDRA results for this assessment can be found in **Appendix D.**

Figure 8 Dalrymple Service Road / Site Access intersection - SIDRA Layout





| Table 14 Dalrymple Service Road / Site Access intersection - SIDRA Summary (AM) | | | | | |
|---|-------|----------------|-----------------|-------------------|--|
| Scenario | DOS | Critical Delay | 95th %ile Queue | Critical Movement | |
| 2025 BG | 0.014 | 5.5 sec | 0.3m | Western Left | |
| 2035 BG | 0.014 | 5.5 sec | 0.3m | Western Left | |
| 2025 BG + DEV | 0.036 | 5.5 sec | 0.7m | Western Left | |
| 2035 BG + DEV | 0.037 | 5.5 sec | 0.7m | Western Left | |

Table 15 Dalrymple Service Road / Site Access intersection - SIDRA Summary (PM)

| Scenario | DOS | Critical Delay | 95th %ile Queue | Critical Movement |
|---------------|-------|----------------|-----------------|--------------------------|
| 2025 BG | 0.038 | 5.5 sec | 0.4m | Western Left |
| 2035 BG | 0.045 | 5.5 sec | 0.4m | Western Left |
| 2025 BG + DEV | 0.063 | 5.5 sec | 1.6m | Western Left |
| 2035 BG + DEV | 0.063 | 5.5 sec | 1.6m | Western Left |

The results presented in Table 8 and Table 9 above suggest that the intersection is within acceptable performance thresholds for a priority-controlled intersection (i.e. DOS less than 0.8) in all scenarios.

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Intersection Performance Summary

All scenarios in both AM and PM peak periods were found to operate within the threshold criteria for each intersection type, as defined in the DTMR Guide to Traffic Impact Assessment (GTIA). This includes a maximum Degree of Saturation (DOS) of 0.90 for signalised intersections, 0.85 for roundabouts, and 0.80 for priority-controlled intersections. No movements exceeded these thresholds, confirming the surrounding network is capable of accommodating the proposed development traffic.

Velocity Response – Item 2b

A swept path assessment has been undertaken for the Toyota Coaster bus which indicates the vehicle can safely and effectively enter the site in a forward gear, drop off students outside of the parking aisle (maintaining more than 6.5m), then depart the site in a forward gear while maintaining adequate 300mm buffers to all obstructions.

These swept paths are provided in **Appendix C**.



1.2 Conclusion

Velocity Traffic believes the parking shortfall is comfortably catered for by the existing parking both on-site and within the available on-street parking area.

The traffic generated by the site is minimal as detailed by the SIDRA assessment and no intersections are expected to have an increase in net delay of more than 5% as detailed in the GTIA.

Author:

Casey Schackow Director

Effective Date 16/06/2025

C. Schackow

Approved By:

Harj Singh Transport Advisor RPEQ 22364 Date Approved

16/06/2025

H.Singh

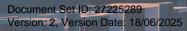




APPENDIX A

INFORMATION REQUEST







Date >> 07 April 2025

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Email >> planning@bespokepd.com.au

Dear Sir/Madam

Information Request Planning Act 2016

As per Council's correspondence on 7 April 2025 please be advised that, upon review of the below mentioned development application, further information is required to undertake a comprehensive assessment. In accordance with section 12 of Development Assessment Rules under the *Planning Act 2016* the following information is requested.

Application Details

The information requested is set out below >>

Request Item 1 - Confirmation of On-Site Parking Requirements

The applicant is requested to provide:

- a) confirmation of the age of students attending the proposed educational establishment;
- b) the number of students able to be collected via shuttle bus; and
- c) details of bus pick-up and set-down for the shuttle bus.

Reason

To demonstrate compliance with Performance Outcome PO2 of the Works code of the Townsville City Plan.

Advice

The applicant is advised that the traffic impact assessment utilises the parking calculation provided in SC6.10 for tertiary education rather than that for secondary education which alters depending on the number of driving age students. Assuming all 128 students attending at one time are of driving age and each of the 30 staff attend each day, there is a shortfall of 24 car parking spaces rather than the 8 detailed in the traffic impact assessment.

As noted in the planning report, the provision of shuttle bus may alleviate this shortfall somewhat, but in order to do so, it must be able to safely enter and exit the site as there are no drop-off facilities explicitly denoted on the plans and Council requires an understanding of how many students will be able to use this service.

Request Item 2 - Traffic Impact Assessment

- a. The applicant is requested to provide an amended Traffic Impact Assessment (TIA) report certified by an RPEQ and which meets the requirements of a Moderate TIA in accordance with SC6.4 Development manual planning scheme policy of the Townsville City Plan. Per SC6.4.5.2(2)(c), "a moderate level TIA Report will be performed using traffic forecasting processes or computerised methods (e.g., Sidra) to estimate and analyse traffic flows on the surrounding road network at predefined time horizons."; and
- b. The amended TIA is also requested to include swept path analysis diagrams for the shuttle bus manoeuvres demonstrating that the shuttle bus can safely manoeuvre on the site and enter and exit in a forward gear.

Reason

To demonstrate that the development will not adversely impact the external transport network in accordance with relevant codes and policy direction.

Advice

The applicant is advised that, as per Table SC6.4.5.2 of the Townsville City Plan, a Moderate TIA report is required where a development results in "new or additional trip generation in a peak hour of 20 to 300 trips directly accessing a street." The submitted TIA identifies that peak hour trip generation falls within this range.

Council prefers an amended TIA document, rather than an addendum to the original report.

Request Item 3 - Water & Sewer Assessment

The applicant is requested to provide a water and sewer assessment report certified by an RPEQ which identifies the demands associated with the development and any upgrades required to Council's infrastructure to accommodate the development.

Reason

To demonstrate that infrastructure is adequate for the needs of the intended use in accordance with relevant codes and policy direction.

Advice

Council's water and sewer network models can be requested via the following link: <u>https://fs6.formsite.com/townsvillecc/form83/index.html</u>.

Council wishes to provide the following advice:

Advice Item 1 - Landscaping

The applicant is advised that Council is implementing development conditions that will require additional plantings, consisting of low shrubs and ground covers to provide dense coverage to the main garden in front of the building to better align with the Landscape code of the Townsville City Plan.

End of Information Request >>

Under the provisions of the Development Assessment Rules under the *Planning Act 2016*, you have three options available in response to this Information Request. You may give the assessment manager (in this instance Council):

- (a) all of the information requested; or
- (b) part of the information requested; or
- (c) a notice that none of the information will be provided.

For any response given in accordance with items (b) and (c) above, you may also advise Council that it must proceed with its assessment of the development application.

Please be aware that under the Development Assessment Rules under the *Planning Act 2016*, the applicant is to respond to any Information Request within 3 months of the request. If you do not respond to the Information Request within this time period, or, within a further period agreed between the applicant and Council, it will be taken that you have decided not to provide a response. In the event of no response being received, Council will continue with the assessment of the application without the information requested.

Council prefers that all of the information requested be submitted as one package. If any additional matters arise as a result of the information submitted, or, as a result of public notification (where applicable), you will be advised accordingly.

Should any referral agency make an information request, you are reminded of your obligation to provide council with a copy of the information response provided to that referral agency.

You may wish to follow the progress of this application using PD Online on Council's website <u>www.townsville.qld.gov.au</u>

If you have any further queries in relation to the above, please do not hesitate to contact Jake Kidner on telephone 07 4417 5240, or email <u>developmentassessment@townsville.qld.gov.au</u>.

Yours faithfully

For Assessment Manager Planning and Development



APPENDIX B

CONCEPT PLANS



Document Set ID: 27225289 Version: 2, Version Date: 18/06/2025

GENERAL SITE NOTES

- SEWERAGE AND/OR SEPTIC TO BE IN ACCORDANCE WITH LOCAL BY-LAWS AND WATER & SEWERAGE ACT AMENDMENT ACT. POSITION OF STORWWATER LINES, DOWNPIPES, RETAINING WALLS, CUT/FILL EMBANKMENTS ARE APPROXIMATE ONLY AND MAY VARY TO SUIT SITE CONDITIONS AND THE BUILDER IS TO
- VERIFY AND ADJUST AS REQUIRED. ALL CUT/FILL EMBANKMENTS, RETAINING WALLS SHOWN AND CONSTRUCTED ARE TO COMPLY WITH COUNCIL POLICY & BCA HOUSING PROVISIONS.
- 4. STORMWATER PIPES TO BE 90 mm CLASS 6 UPVC & LAID IN ACCORDANCE WITH BCA HOUSING PROVISIONS UNLESS SPECIFIED ELSEWHERE. ONE 90 mm UPVC PIPE PER 100 SQM OF ROOF AREA LAID TO 1:100 MIN GRADE. PAD CUT TO ALLOW GROUND WATER TO DRAIN AWAY FROM
- DWELLING ALL ROUND AT 1:20 FALL.
 ALL BOUNDARY CLEARANCES AND SET OUT DIMENSIONS TO BE VERIFIED PRIOR TO COMMENCEMENT OF WORK.
 THE BUILDER IS TO VERIFY ALL DIMENSIONS AND LEVELS ON
- PLAN PRIOR TO COMMENCEMENT OF THE JOB AS NO RESPONSIBILITY IS TAKEN AFTER WORK HAS COMMENCED.

TO BE POSITIONED BY BUILDER

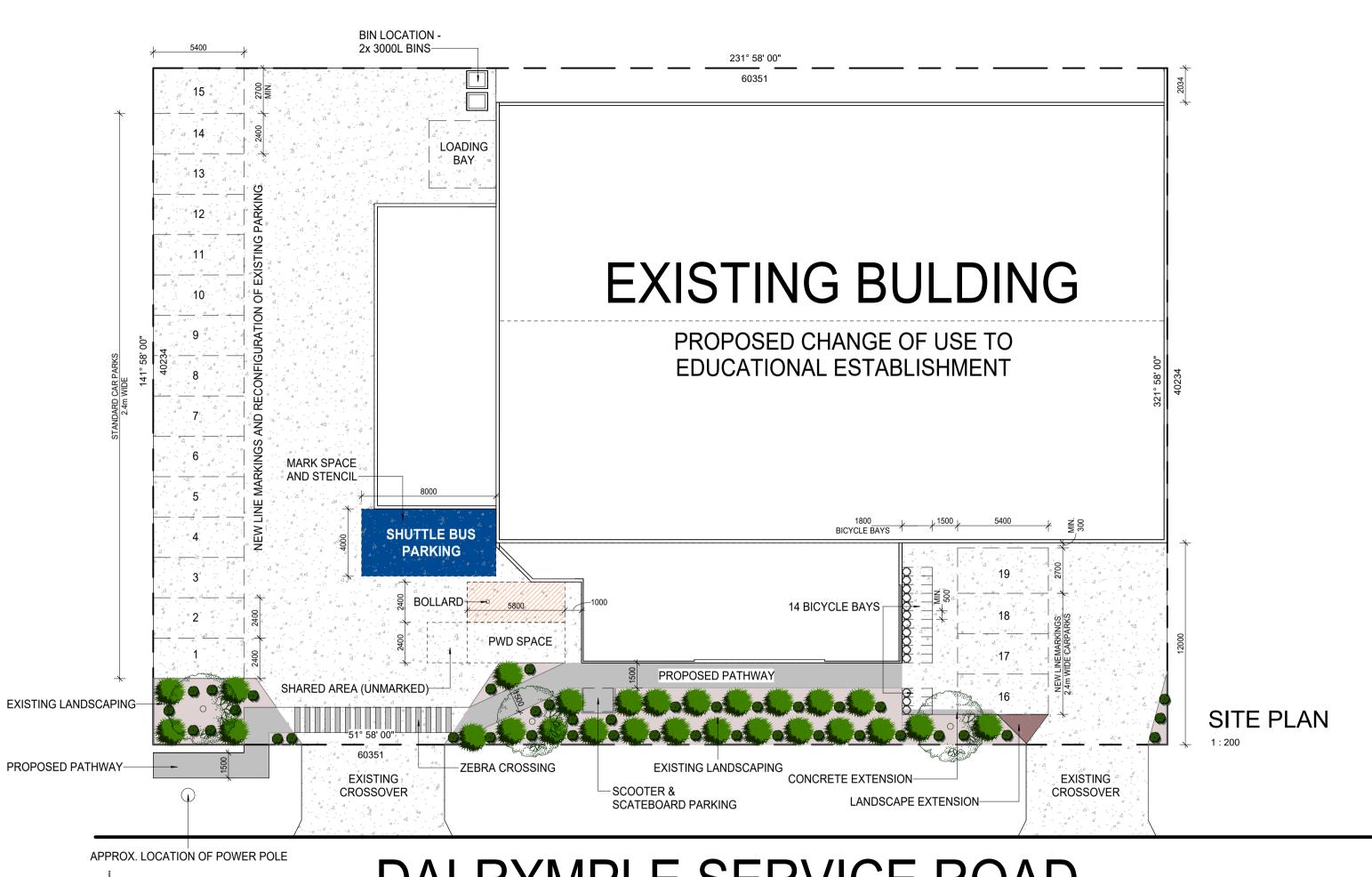
| IF . | IF APPLICABLE (IF INCLUDED IN SPEC.) | | | | | |
|------|--------------------------------------|---|-----------------|--|--|--|
| • | METER BOX | • | WATER TANK | | | |
| • | HOTWATER SYSTEM | • | RETAINING WALLS | | | |
| • | CLOTHES LINE | • | FENCES - GATES | | | |
| • | LETTERBOX | • | DRIVEWAY - PATH | | | |

LINE TYPES

| CUT / FILL RETAINING WALL BATTERS BOUNDARY STEEL BEAM | |
|---|--|
| LEGEND | |

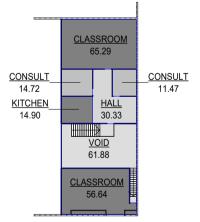
| WW | WATER PIPE |
|----------------|------------------------|
| —_S—S—S—_ | SEWER PIPE |
| —SW—SW—SW— | STORMWATER PIPE |
| | STORMWATER PIT |
| (\mathbf{H}) | WATER HYDRANT |
| Ŀ | SEWER HOUSE CONNECTION |
| \odot | SEWER PIT |
| | |

| SITE AREA SITE COVER | 2428 m² REMAIN UNCHANGED | |
|-------------------------|-----------------------------|---------|
| AREA SCHI | EDULE | |
| LEVEL | NAME | AREA |
| GROUND FLOOR | AMENITY | 55.96 |
| GROUND FLOOR | CLASSROOM | 409.25 |
| GROUND FLOOR | CONSULT | 82.05 |
| GROUND FLOOR | KEEP | 22.99 |
| GROUND FLOOR | OFFICE | 22.14 |
| GROUND FLOOR | RECEPTION | 55.39 |
| GROUND FLOOR | SICK | 17.87 |
| GROUND FLOOR | STAFF | 61.93 |
| GROUND FLOOR | STUDENT BREAKOUT | 427.07 |
| GROUND FLOOR | VOCATIONAL TRAINING | 169.39 |
| FIRST FLOOR | CLASSROOM | 121.93 |
| FIRST FLOOR | CONSULT | 26.19 |
| FIRST FLOOR | HALL | 30.33 |
| FIRST FLOOR | KITCHEN | 14.90 |
| FIRST FLOOR | VOID | 61.88 |
| Grand total | | 1579.27 |



<u>CONSULT</u> 42.90 VOCATIONAL TRAINING 169.39 CLASSROOM 141.43 <u>CONSULT</u> 39.14 AMENITY 16.45 <u>KEEP</u> STUDENT BREAKOUT 22.99 427.07 . 」 . Ц . SICK 17.87 STAFF **CLASSROOM** 59.46 61.93 TITT AMENITY CLASSROOM 135,39 39.51





DALRYMPLE SERVICE ROAD

DATE REVISION DESCRIPTION 25/02/25 CHANGE OF USE CONCEPT PLAN 27/05/25 RESPONSE TO IR

DRAWING NAM

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<u>CLIENT</u> BUSY SCHOOLS <u>SITE ADDRESS</u> 153 DALRYMPLE ROAD - GARBUTT REAL PROPERTY DESCRIPTION

<u>SCALE</u> As indicated @ A2 <u>wind</u> TBA <u>SHEET #</u> 02 <u>ЈОВ #</u> 25703

PRELIMINARY DRAWING - NOT FOR CONSTRUCTION

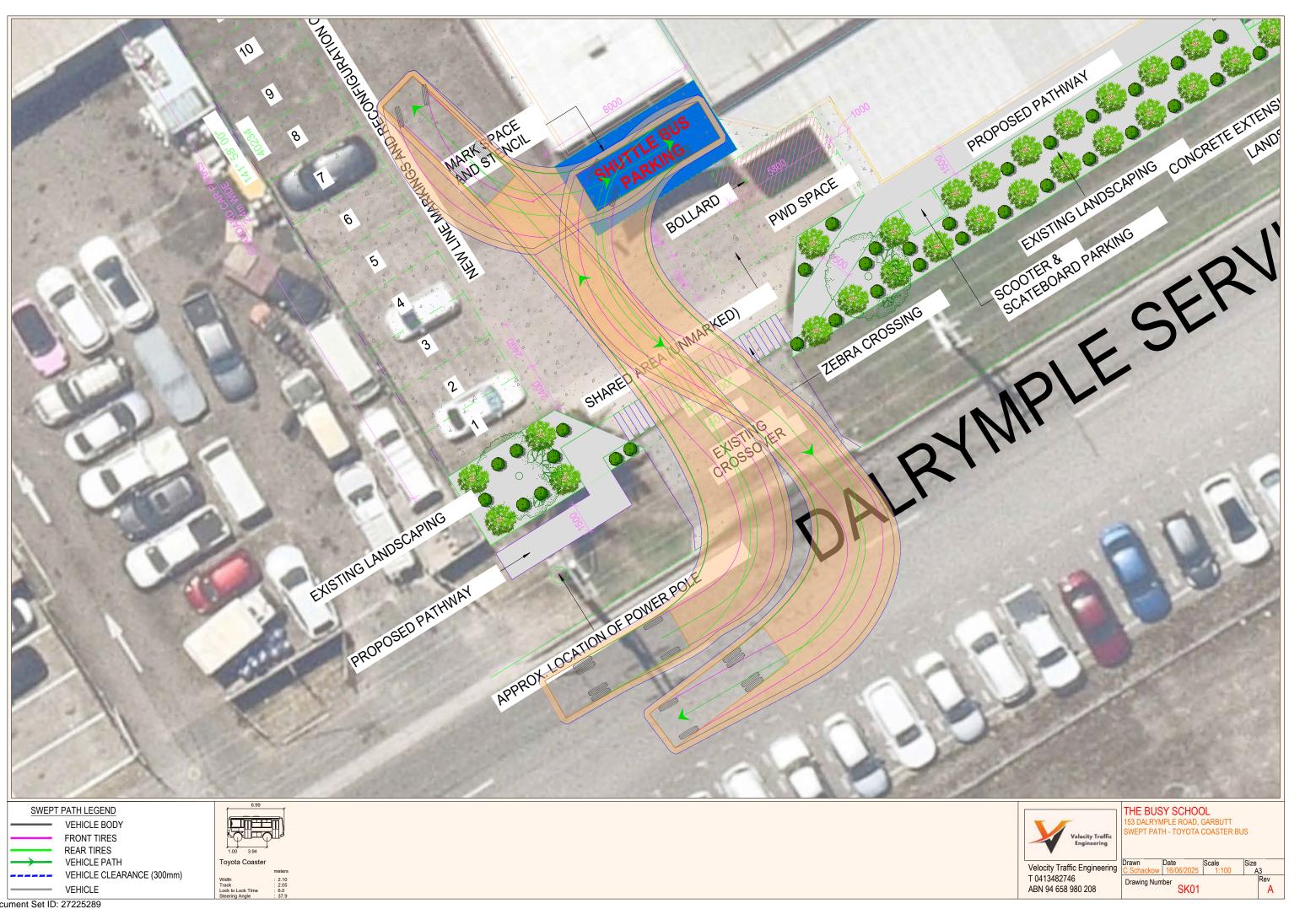


APPENDIX C

SWEPT PATH ASSESSMENT



Document Set ID: 27225289 Version: 2, Version Date: 18/06/2025



Document Set ID: 27225289 Version: 2, Version Date: 18/06/2025

VEHICLE

| • | |
|--------------------------|------|
| Velocity Traffic Enginee | ring |
| T 0413482746 | |
| ABN 94 658 980 208 | |
| | |

Drawing Number

Α

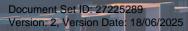
SK01





APPENDIX D SIDRA ASSESSMENT

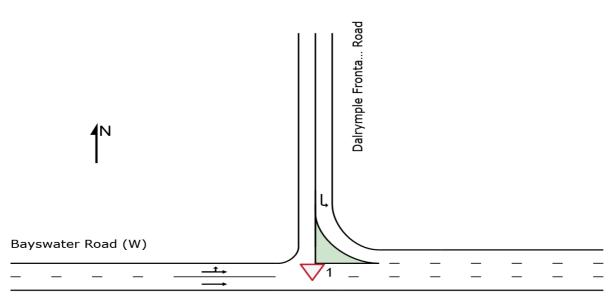




SITE LAYOUT Site: [1] 2025 BG AM - Dalrymple Frontage / Bayswater (General)

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



Bayswater Road (E)

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Site: [1] 2025 BG AM - Dalrymple Frontage / Bayswater (General)

Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

| Vehicle Movement Performance | | | | | | | | | | | | | |
|--------------------------------|--------|-----------|--------------|--------------|-------|-------|----------|-------|--------|------------|----------------|--------|-------|
| Mov | Turn | Mov | Demand | Arrival | Deg. | Aver. | Level of | 95% B | ack Of | Prop. Eff. | | Number | Aver. |
| ID | | Class | Flows | Flows | Satn | Delay | Service | Qu | eue | Qued | Stop of Cycles | | Speed |
| | | | [Total HV] | [Total HV] | | | | [Veh. | Dist] | | Rate to Depart | | |
| | | | veh/h % | veh/h % | v/c | sec | | veh | m | | | | km/h |
| North: Dalrymple Frontage Road | | | | | | | | | | | | | |
| 7 | L2 | All MCs | 8 12.0 | 8 12.0 | 0.005 | 4.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.51 | 0.00 | 33.0 |
| Appro | bach | | 8 12.0 | 8 12.0 | 0.005 | 4.1 | NA | 0.0 | 0.0 | 0.00 | 0.51 | 0.00 | 33.0 |
| West: | Bays | water Roa | ad (W) | | | | | | | | | | |
| 10 | L2 | All MCs | 4 12.0 | 4 12.0 | 0.230 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 44.2 |
| 11 | T1 | All MCs | 834 11.0 | 834 11.0 | 0.230 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.7 |
| Appro | bach | | 838 11.0 | 838 11.0 | 0.230 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.6 |
| All Ve | hicles | | 846 11.0 | 846 11.0 | 0.230 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 59.4 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: [2] 2025 BG PM - Dalrymple Frontage / Bayswater (General)

Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

| Vehicle Movement Performance | | | | | | | | | | | | | |
|--------------------------------|--------|-----------|--------------|--------------|-------|-------|----------|-------|--------|-------|----------------|----------|-------|
| Mov | Turn | Mov | Demand | Arrival | Deg. | Aver. | Level of | 95% B | ack Of | Prop. | Eff. | Number | Aver. |
| ID | | Class | Flows | Flows | Satn | Delay | Service | Qu | eue | Qued | Stop o | f Cycles | Speed |
| | | | [Total HV] | [Total HV] | | | | [Veh. | Dist] | | Rate to Depart | | |
| | | | veh/h % | veh/h % | v/c | sec | | veh | m | | | | km/h |
| North: Dalrymple Frontage Road | | | | | | | | | | | | | |
| 7 | L2 | All MCs | 63 12.0 | 63 12.0 | 0.037 | 4.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.51 | 0.00 | 33.0 |
| Appro | bach | | 63 12.0 | 63 12.0 | 0.037 | 4.1 | NA | 0.0 | 0.0 | 0.00 | 0.51 | 0.00 | 33.0 |
| West: | Bays | water Roa | ad (W) | | | | | | | | | | |
| 10 | L2 | All MCs | 1 12.0 | 1 12.0 | 0.181 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 44.3 |
| 11 | T1 | All MCs | 659 11.0 | 659 11.0 | 0.181 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| Appro | bach | | 660 11.0 | 660 11.0 | 0.181 | 0.0 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.8 |
| All Ve | hicles | | 723 11.1 | 723 11.1 | 0.181 | 0.4 | NA | 0.0 | 0.0 | 0.00 | 0.05 | 0.00 | 57.4 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: [3] 2025 BG AM + DEV - Dalrymple Frontage / Bayswater (General)

Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

| Vehicle Movement Performance | | | | | | | | | | | | | |
|--------------------------------|--------|-----------|--------------|--------------|-------|-------|----------|-------|---------------|------|----------------|----------|-------|
| Mov | Turn | Mov | Demand | Arrival | Deg. | Aver. | Level of | 95% B | 95% Back Of F | | Eff. | Number | Aver. |
| ID | | Class | Flows | Flows | Satn | Delay | Service | Qu | eue | Qued | Stop o | f Cycles | Speed |
| | | | [Total HV] | [Total HV] | | | | [Veh. | Dist] | | Rate to Depart | | |
| | | | veh/h % | veh/h % | v/c | sec | | veh | m | | | | km/h |
| North: Dalrymple Frontage Road | | | | | | | | | | | | | |
| 7 | L2 | All MCs | 17 12.0 | 17 12.0 | 0.010 | 4.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.51 | 0.00 | 33.0 |
| Appro | ach | | 17 12.0 | 17 12.0 | 0.010 | 4.1 | NA | 0.0 | 0.0 | 0.00 | 0.51 | 0.00 | 33.0 |
| West: | Bays | water Roa | ad (W) | | | | | | | | | | |
| 10 | L2 | All MCs | 53 12.0 | 53 12.0 | 0.244 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.07 | 0.00 | 42.8 |
| 11 | T1 | All MCs | 834 11.0 | 834 11.0 | 0.244 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.03 | 0.00 | 58.4 |
| Approach | | 886 11.1 | 886 11.1 | 0.244 | 0.4 | NA | 0.0 | 0.0 | 0.00 | 0.04 | 0.00 | 57.1 | |
| All Ve | hicles | | 903 11.1 | 903 11.1 | 0.244 | 0.4 | NA | 0.0 | 0.0 | 0.00 | 0.04 | 0.00 | 56.7 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: [4] 2025 BG PM + DEV - Dalrymple Frontage / Bayswater (General)

Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

| Vehicle Movement Performance | | | | | | | | | | | | | |
|--------------------------------|--------|-----------|--------------|--------------|-------|-------|----------|-------|-----------------|------|----------------|--------|-------|
| Mov | Turn | Mov | Demand | Arrival | Deg. | Aver. | Level of | 95% B | % Back Of Prop. | | Eff. | Number | Aver. |
| ID | | Class | Flows | Flows | Satn | Delay | Service | Qu | eue | Qued | Stop of Cycles | | Speed |
| | | | [Total HV] | [Total HV] | | | | [Veh. | Dist] | | Rate to Depart | | |
| | | | veh/h % | veh/h % | v/c | sec | | veh | m | | | | km/h |
| North: Dalrymple Frontage Road | | | | | | | | | | | | | |
| 7 | L2 | All MCs | 96 12.0 | 96 12.0 | 0.056 | 4.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.51 | 0.00 | 33.0 |
| Appro | ach | | 96 12.0 | 96 12.0 | 0.056 | 4.1 | NA | 0.0 | 0.0 | 0.00 | 0.51 | 0.00 | 33.0 |
| West: | Bays | water Roa | ad (W) | | | | | | | | | | |
| 10 | L2 | All MCs | 13 12.0 | 13 12.0 | 0.185 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.02 | 0.00 | 43.8 |
| 11 | T1 | All MCs | 659 11.0 | 659 11.0 | 0.185 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 59.4 |
| Appro | ach | | 672 11.0 | 672 11.0 | 0.185 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 0.00 | 59.0 |
| All Ve | hicles | | 767 11.1 | 767 11.1 | 0.185 | 0.6 | NA | 0.0 | 0.0 | 0.00 | 0.07 | 0.00 | 55.7 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

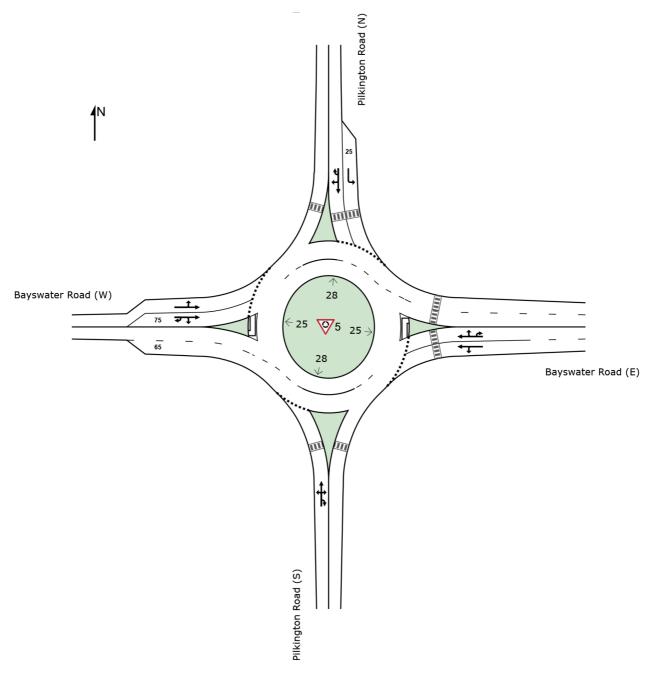
Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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New Site Site Category: (None) Roundabout Site Scenario: 1 | Local Volumes

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



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Site: [5] 2025 BG AM - Bayswater / Pilkington (General) Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Roundabout Site Scenario: 1 | Local Volumes

| Vehi | cle Mo | ovement | Performa | nce | | | | | | | | | |
|--------|----------|-----------|--------------|-----------|-------|-------|----------|-------|--------|-----------------|---------|----------|-------|
| Mov | Turn | Mov | Demand | Arrival | Deg. | Aver. | Level of | 95% B | ack Of | Prop. Eff. Numb | | Number | Aver. |
| ID | | Class | Flows | Flows | Satn | Delay | Service | Que | eue | Qued | Stop o | f Cycles | Speed |
| | | | [Total HV] | | | | | [Veh. | Dist] | | Rate to | o Depart | |
| | | | | veh/h % | v/c | sec | | veh | m | | | | km/h |
| | | ngton Roa | . , | | | | | | | | | | |
| 1 | | All MCs | 58 9.0 | 58 9.0 | 0.233 | 8.5 | LOS A | 1.2 | 8.8 | 0.66 | 0.73 | 0.66 | 47.6 |
| 2 | T1 | All MCs | 27 9.0 | 27 9.0 | 0.233 | 7.4 | LOS A | 1.2 | 8.8 | 0.66 | 0.73 | 0.66 | 47.9 |
| 3 | R2 | | 82 9.0 | 82 9.0 | 0.233 | 12.9 | LOS B | 1.2 | 8.8 | 0.66 | 0.73 | 0.66 | 39.1 |
| 3u | U | All MCs | 1 9.0 | 1 9.0 | 0.233 | 15.1 | LOS B | 1.2 | 8.8 | 0.66 | 0.73 | 0.66 | 40.1 |
| Appro | bach | | 168 9.0 | 168 9.0 | 0.233 | 10.5 | LOS B | 1.2 | 8.8 | 0.66 | 0.73 | 0.66 | 44.3 |
| East: | Baysv | vater Roa | d (E) | | | | | | | | | | |
| 4 | L2 | All MCs | 73 14.0 | 73 14.0 | 0.205 | 5.6 | LOS A | 1.0 | 7.6 | 0.40 | 0.52 | 0.40 | 43.2 |
| 5 | T1 | All MCs | 421 14.0 | 421 14.0 | 0.413 | 5.4 | LOS A | 2.5 | 19.6 | 0.42 | 0.56 | 0.42 | 49.8 |
| 6 | R2 | All MCs | 202 14.0 | 202 14.0 | 0.413 | 10.4 | LOS B | 2.5 | 19.6 | 0.43 | 0.58 | 0.43 | 47.8 |
| 6u | U | All MCs | 1 14.0 | 1 14.0 | 0.413 | 12.7 | LOS B | 2.5 | 19.6 | 0.43 | 0.58 | 0.43 | 41.1 |
| Appro | bach | | 697 14.0 | 697 14.0 | 0.413 | 6.9 | LOS A | 2.5 | 19.6 | 0.42 | 0.56 | 0.42 | 48.7 |
| North | : Pilkir | ngton Roa | id (N) | | | | | | | | | | |
| 7 | L2 | All MCs | 195 13.0 | 195 13.0 | 0.208 | 5.8 | LOS A | 0.9 | 7.1 | 0.55 | 0.63 | 0.55 | 49.4 |
| 8 | T1 | All MCs | 23 13.0 | 23 13.0 | 0.201 | 6.1 | LOS A | 0.8 | 6.6 | 0.56 | 0.73 | 0.56 | 46.6 |
| 9 | R2 | All MCs | 137 13.0 | 137 13.0 | 0.201 | 11.5 | LOS B | 0.8 | 6.6 | 0.56 | 0.73 | 0.56 | 49.5 |
| 9u | U | All MCs | 1 13.0 | 1 13.0 | 0.201 | 13.8 | LOS B | 0.8 | 6.6 | 0.56 | 0.73 | 0.56 | 49.4 |
| Appro | bach | | 356 13.0 | 356 13.0 | 0.208 | 8.0 | LOS A | 0.9 | 7.1 | 0.55 | 0.68 | 0.55 | 49.3 |
| West | Bays | water Roa | ad (W) | | | | | | | | | | |
| 10 | L2 | All MCs | 152 12.0 | 152 12.0 | 0.293 | 5.6 | LOS A | 1.9 | 14.8 | 0.53 | 0.52 | 0.53 | 52.5 |
| 11 | T1 | All MCs | 449 12.0 | 449 12.0 | 0.293 | 5.6 | LOS A | 1.9 | 14.8 | 0.54 | 0.54 | 0.54 | 49.2 |
| 12 | R2 | All MCs | 55 12.0 | 55 12.0 | 0.293 | 11.2 | LOS B | 1.8 | 14.2 | 0.55 | 0.55 | 0.55 | 47.8 |
| 12u | U | All MCs | 1 12.0 | 1 12.0 | 0.293 | 13.5 | LOS B | 1.8 | 14.2 | 0.55 | 0.55 | 0.55 | 51.3 |
| Appro | bach | | 657 12.0 | 657 12.0 | 0.293 | 6.1 | LOS A | 1.9 | 14.8 | 0.54 | 0.54 | 0.54 | 50.1 |
| All Ve | hicles | | 1878 12.7 | 1878 12.7 | 0.413 | 7.2 | LOS A | 2.5 | 19.6 | 0.51 | 0.59 | 0.51 | 49.0 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Site: [6] 2025 BG PM - Bayswater / Pilkington (General)

Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Roundabout Site Scenario: 1 | Local Volumes

| Vehic | cle Mo | ovement | t Performa | nce | | | | | | | | | |
|-----------|----------|--------------|--------------------------|-------------------------|--------------|----------------|---------------------|---------------|------------|---------------|--------|---------------------|----------------|
| Mov ID | Turn | Mov Class | Demand Flows | Arrival Flows | Deg. Satn | Aver. Delay | Level of Service | 95% B Que | | Prop. Qued | | Number of Cycles | Aver. Speed |
| | | | [Total HV] veh/h % | [Total HV] veh/h % | v/c | sec | | [Veh. veh | Dist] m | | Rate t | o Depart | km/h |
| South | : Pilkir | ngton Roa | ad (S) | | | | | | | | | | |
| 1 | L2 | All MCs | 7 9.0 | 7 9.0 | 0.022 | 7.2 | LOS A | 0.1 | 0.7 | 0.53 | 0.64 | 0.53 | 48.5 |
| 2 | T1 | All MCs | 1 9.0 | 1 9.0 | 0.022 | 6.1 | LOS A | 0.1 | 0.7 | 0.53 | 0.64 | 0.53 | 48.9 |
| 3 | R2 | All MCs | 8 9.0 | 8 9.0 | 0.022 | 11.5 | LOS B | 0.1 | 0.7 | 0.53 | 0.64 | 0.53 | 40.2 |
| 3u | U | All MCs | 1 9.0 | 1 9.0 | 0.022 | 13.8 | LOS B | 0.1 | 0.7 | 0.53 | 0.64 | 0.53 | 41.2 |
| Appro | ach | | 18 9.0 | 18 9.0 | 0.022 | 9.6 | LOS A | 0.1 | 0.7 | 0.53 | 0.64 | 0.53 | 45.0 |
| East: | Baysv | vater Roa | d (E) | | | | | | | | | | |
| 4 | L2 | All MCs | 61 14.0 | 61 14.0 | 0.143 | 6.2 | LOS A | 0.6 | 5.0 | 0.44 | 0.56 | 0.44 | 42.7 |
| 5 | T1 | All MCs | 337 14.0 | 337 14.0 | 0.287 | 5.7 | LOS A | 1.5 | 11.9 | 0.46 | 0.56 | 0.46 | 50.2 |
| 6 | R2 | All MCs | 45 14.0 | 45 14.0 | 0.287 | 10.8 | LOS B | 1.5 | 11.9 | 0.46 | 0.56 | 0.46 | 48.5 |
| 6u | U | All MCs | 1 14.0 | 1 14.0 | 0.287 | 13.1 | LOS B | 1.5 | 11.9 | 0.46 | 0.56 | 0.46 | 42.0 |
| Appro | ach | | 444 14.0 | 444 14.0 | 0.287 | 6.3 | LOS A | 1.5 | 11.9 | 0.45 | 0.56 | 0.45 | 49.3 |
| North | : Pilkir | igton Roa | id (N) | | | | | | | | | | |
| 7 | L2 | All MCs | 224 13.0 | 224 13.0 | 0.254 | 5.9 | LOS A | 1.0 | 7.9 | 0.51 | 0.63 | 0.51 | 49.6 |
| 8 | T1 | All MCs | 37 13.0 | 37 13.0 | 0.242 | 5.4 | LOS A | 1.0 | 7.6 | 0.49 | 0.69 | 0.49 | 46.9 |
| 9 | R2 | All MCs | 205 13.0 | 205 13.0 | 0.242 | 10.9 | LOS B | 1.0 | 7.6 | 0.49 | 0.69 | 0.49 | 49.7 |
| 9u | U | All MCs | 1 13.0 | 1 13.0 | 0.242 | 13.1 | LOS B | 1.0 | 7.6 | 0.49 | 0.69 | 0.49 | 49.7 |
| Appro | ach | | 467 13.0 | 467 13.0 | 0.254 | 8.1 | LOS A | 1.0 | 7.9 | 0.50 | 0.66 | 0.50 | 49.4 |
| West: | Bays | water Roa | ad (W) | | | | | | | | | | |
| 10 | L2 | All MCs | 55 12.0 | 55 12.0 | 0.197 | 4.2 | LOS A | 1.2 | 9.5 | 0.21 | 0.38 | 0.21 | 53.9 |
| 11 | T1 | All MCs | 438 12.0 | 438 12.0 | 0.197 | 4.0 | LOS A | 1.2 | 9.5 | 0.22 | 0.41 | 0.22 | 51.0 |
| 12 | R2 | All MCs | 73 12.0 | 73 12.0 | 0.197 | 9.4 | LOS A | 1.2 | 9.3 | 0.22 | 0.45 | 0.22 | 49.1 |
| 12u | U | All MCs | 1 12.0 | 1 12.0 | 0.197 | 11.7 | LOS B | 1.2 | 9.3 | 0.22 | 0.45 | 0.22 | 52.4 |
| Appro | ach | | 566 12.0 | 566 12.0 | 0.197 | 4.7 | LOS A | 1.2 | 9.5 | 0.22 | 0.41 | 0.22 | 51.1 |
| All Ve | hicles | | 1496 12.9 | 1496 12.9 | 0.287 | 6.3 | LOS A | 1.5 | 11.9 | 0.38 | 0.54 | 0.38 | 50.0 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Site: [7] 2025 BG AM + DEV - Bayswater / Pilkington (General)

Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Roundabout Site Scenario: 1 | Local Volumes

| 1.4 | | | | A | | A | 1 | | | | | | A |
|-------|----------|-----------|-----------|-----------------------|-------|-------|----------|---------------|---------------|-------|------|--------------------|-------|
| Mov | lurn | Mov | Demand | Arrival | Deg. | Aver. | Level of | | ack Of | Prop. | | Number | Aver. |
| ID | | Class | Flows | Flows [Total HV] | Satn | Delay | Service | Qui [Veh. | eue Dist] | Qued | | f Cycles Depart | Speed |
| | | | | veh/h % | v/c | sec | | veh | m | | | Depart | km/h |
| South | : Pilkir | ngton Roa | | | | | | | | | | | |
| 1 | L2 | All MCs | 58 9.0 | 58 9.0 | 0.262 | 8.7 | LOS A | 1.3 | 10.1 | 0.67 | 0.74 | 0.67 | 47.2 |
| 2 | T1 | All MCs | 27 9.0 | 27 9.0 | 0.262 | 7.6 | LOS A | 1.3 | 10.1 | 0.67 | 0.74 | 0.67 | 47.6 |
| 3 | R2 | All MCs | 102 9.0 | 102 9.0 | 0.262 | 13.0 | LOS B | 1.3 | 10.1 | 0.67 | 0.74 | 0.67 | 38.7 |
| 3u | U | All MCs | 1 9.0 | 1 9.0 | 0.262 | 15.2 | LOS B | 1.3 | 10.1 | 0.67 | 0.74 | 0.67 | 39.7 |
| Appro | ach | | 188 9.0 | 188 9.0 | 0.262 | 10.9 | LOS B | 1.3 | 10.1 | 0.67 | 0.74 | 0.67 | 43.5 |
| East: | Baysw | vater Roa | d (E) | | | | | | | | | | |
| 4 | L2 | All MCs | 73 14.0 | 73 14.0 | 0.209 | 5.6 | LOS A | 1.0 | 7.8 | 0.40 | 0.52 | 0.40 | 43.2 |
| 5 | T1 | All MCs | 421 14.0 | 421 14.0 | 0.421 | 5.5 | LOS A | 2.6 | 20.2 | 0.43 | 0.57 | 0.43 | 49.7 |
| 6 | R2 | All MCs | 202 14.0 | 202 14.0 | 0.421 | 10.4 | LOS B | 2.6 | 20.2 | 0.44 | 0.58 | 0.44 | 47.7 |
| 6u | U | All MCs | 13 14.0 | 13 14.0 | 0.421 | 12.7 | LOS B | 2.6 | 20.2 | 0.44 | 0.58 | 0.44 | 41.0 |
| Appro | ach | | 708 14.0 | 708 14.0 | 0.421 | 7.0 | LOS A | 2.6 | 20.2 | 0.43 | 0.57 | 0.43 | 48.6 |
| North | : Pilkin | igton Roa | id (N) | | | | | | | | | | |
| 7 | L2 | All MCs | 199 13.0 | 199 13.0 | 0.217 | 5.9 | LOS A | 1.0 | 7.6 | 0.57 | 0.65 | 0.57 | 49.3 |
| 8 | T1 | All MCs | 23 13.0 | 23 13.0 | 0.207 | 6.3 | LOS A | 0.9 | 6.9 | 0.58 | 0.74 | 0.58 | 46.5 |
| 9 | R2 | All MCs | 137 13.0 | 137 13.0 | 0.207 | 11.7 | LOS B | 0.9 | 6.9 | 0.58 | 0.74 | 0.58 | 49.4 |
| 9u | U | All MCs | 1 13.0 | 1 13.0 | 0.207 | 13.9 | LOS B | 0.9 | 6.9 | 0.58 | 0.74 | 0.58 | 49.3 |
| Appro | ach | | 360 13.0 | 360 13.0 | 0.217 | 8.2 | LOS A | 1.0 | 7.6 | 0.57 | 0.69 | 0.57 | 49.2 |
| West: | Bays | water Roa | ad (W) | | | | | | | | | | |
| 10 | L2 | All MCs | 152 12.0 | 152 12.0 | 0.305 | 5.9 | LOS A | 2.0 | 15.7 | 0.56 | 0.54 | 0.56 | 52.4 |
| 11 | T1 | All MCs | 461 12.0 | 461 12.0 | 0.305 | 5.9 | LOS A | 2.0 | 15.7 | 0.57 | 0.55 | 0.57 | 49.1 |
| 12 | R2 | All MCs | 55 12.0 | 55 12.0 | 0.305 | 11.4 | LOS B | 1.9 | 14.9 | 0.58 | 0.57 | 0.58 | 47.6 |
| 12u | U | All MCs | 1 12.0 | 1 12.0 | 0.305 | 13.7 | LOS B | 1.9 | 14.9 | 0.58 | 0.57 | 0.58 | 51.2 |
| Appro | ach | | 668 12.0 | 668 12.0 | 0.305 | 6.3 | LOS A | 2.0 | 15.7 | 0.57 | 0.55 | 0.57 | 49.9 |
| | hicles | | 1925 12.6 | 1925 12.6 | 0.421 | 7.4 | LOS A | 2.6 | 20.2 | 0.53 | 0.60 | 0.53 | 48.8 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

Site: [8] 2025 BG PM + DEV - Bayswater / Pilkington (General)

Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Roundabout Site Scenario: 1 | Local Volumes

| Vehio | cle M | ovemen | t Performa | | | | | | | | | | |
|--------|----------|-----------|-----------------|-----------|-------|-------|----------|-------|--------|-------|---------|----------|----------|
| Mov | Turn | Mov | Demand | Arrival | Deg. | Aver. | Level of | 95% B | | Prop. | | Number | Aver. |
| ID | | Class | Flows | Flows | Satn | Delay | Service | Que | | Qued | | f Cycles | Speed |
| | | | [Total HV] | | | | | [Veh. | Dist] | | Rate to | Depart | Luna /la |
| 0 | | | | veh/h % | v/c | sec | _ | veh | m | _ | _ | _ | km/h |
| | | ngton Roa | ad (S) 7 9.0 | 7.00 | 0.029 | 7.0 | LOS A | 0.4 | 0.0 | 0.53 | 0.66 | 0.53 | 40.0 |
| 1 | | All MCs | | 7 9.0 | | 7.3 | - | 0.1 | 0.9 | | | | 48.0 |
| 2 | T1 | All MCs | 1 9.0 | 1 9.0 | 0.029 | 6.1 | LOS A | 0.1 | 0.9 | 0.53 | 0.66 | 0.53 | 48.4 |
| 3 | R2 | - | 14 9.0 | 14 9.0 | 0.029 | 11.6 | LOS B | 0.1 | 0.9 | 0.53 | 0.66 | 0.53 | 39.8 |
| 3u | U | All MCs | 1 9.0 | 1 9.0 | 0.029 | 13.8 | LOS B | 0.1 | 0.9 | 0.53 | 0.66 | 0.53 | 40.7 |
| Appro | ach | | 23 9.0 | 23 9.0 | 0.029 | 10.1 | LOS B | 0.1 | 0.9 | 0.53 | 0.66 | 0.53 | 43.6 |
| East: | Baysv | vater Roa | d (E) | | | | | | | | | | |
| 4 | L2 | All MCs | 61 14.0 | 61 14.0 | 0.143 | 6.2 | LOS A | 0.6 | 5.1 | 0.44 | 0.56 | 0.44 | 42.6 |
| 5 | T1 | All MCs | 337 14.0 | 337 14.0 | 0.289 | 5.7 | LOS A | 1.5 | 12.0 | 0.46 | 0.56 | 0.46 | 50.1 |
| 6 | R2 | All MCs | 45 14.0 | 45 14.0 | 0.289 | 10.8 | LOS B | 1.5 | 12.0 | 0.46 | 0.56 | 0.46 | 48.5 |
| 6u | U | All MCs | 3 14.0 | 3 14.0 | 0.289 | 13.1 | LOS B | 1.5 | 12.0 | 0.46 | 0.56 | 0.46 | 41.9 |
| Appro | ach | | 446 14.0 | 446 14.0 | 0.289 | 6.4 | LOS A | 1.5 | 12.0 | 0.45 | 0.56 | 0.45 | 49.2 |
| North | : Pilkir | ngton Roa | d (N) | | | | | | | | | | |
| 7 | L2 | All MCs | 225 13.0 | 225 13.0 | 0.257 | 6.0 | LOS A | 1.0 | 8.0 | 0.52 | 0.63 | 0.52 | 49.6 |
| 8 | T1 | All MCs | 37 13.0 | 37 13.0 | 0.243 | 5.5 | LOS A | 1.0 | 7.7 | 0.50 | 0.70 | 0.50 | 46.9 |
| 9 | R2 | All MCs | 205 13.0 | 205 13.0 | 0.243 | 10.9 | LOS B | 1.0 | 7.7 | 0.50 | 0.70 | 0.50 | 49.7 |
| 9u | U | All MCs | 1 13.0 | 1 13.0 | 0.243 | 13.2 | LOS B | 1.0 | 7.7 | 0.50 | 0.70 | 0.50 | 49.7 |
| Appro | ach | | 468 13.0 | 468 13.0 | 0.257 | 8.1 | LOS A | 1.0 | 8.0 | 0.51 | 0.66 | 0.51 | 49.4 |
| West: | Bays | water Roa | ad (W) | | | | | | | | | | |
| 10 | L2 | All MCs | 55 12.0 | 55 12.0 | 0.200 | 4.3 | LOS A | 1.2 | 9.5 | 0.22 | 0.38 | 0.22 | 53.8 |
| 11 | T1 | All MCs | 441 12.0 | 441 12.0 | 0.200 | 4.0 | LOS A | 1.2 | 9.5 | 0.23 | 0.41 | 0.23 | 50.9 |
| 12 | R2 | All MCs | 73 12.0 | 73 12.0 | 0.200 | 9.5 | LOS A | 1.2 | 9.3 | 0.24 | 0.45 | 0.24 | 49.1 |
| 12u | U | All MCs | 1 12.0 | 1 12.0 | 0.200 | 11.7 | LOS B | 1.2 | 9.3 | 0.24 | 0.45 | 0.24 | 52.4 |
| Appro | ach | | 569 12.0 | 569 12.0 | 0.200 | 4.8 | LOS A | 1.2 | 9.5 | 0.23 | 0.41 | 0.23 | 51.1 |
| All Ve | hicles | | 1507 12.9 | 1507 12.9 | 0.289 | 6.4 | LOS A | 1.5 | 12.0 | 0.39 | 0.54 | 0.39 | 49.9 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

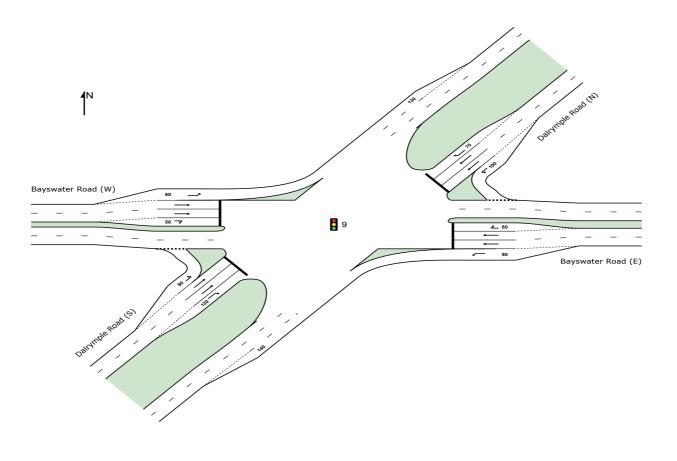
HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

SITE LAYOUT Site: [9] 2025 BG AM - Dalrymple / Bayswater (General)

New Site Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Isolated Site Scenario: 1 | Local Volumes

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



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Site: [9] 2025 BG AM - Dalrymple / Bayswater (General)

Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 131.0 seconds (Site User-Given Phase Times) Site Scenario: 1 | Local Volumes

| Vehi | cle Mo | ovement | t Perfo | rma | nce | | | | | | | | | | |
|--------|---------|-----------|---------|------|---------|-------|--------|-------|----------|-------|--------|-------|---------|--------|-------|
| Mov | Turn | Mov | Dem | nand | Ar | rival | Deg. | Aver. | Level of | 95% B | ack Of | Prop. | Eff. I | Number | Aver. |
| ID | | Class | F | ows | FI | ows | Satn | Delay | Service | Que | eue | Qued | Stop of | Cycles | Speed |
| | | | [Total | HV] | [Total | HV] | | | | [Veh. | Dist] | | Rate to | Depart | |
| | | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | | km/h |
| East: | Baysv | vater Roa | d (E) | | | | | | | | | | | | |
| 4a | L1 | All MCs | 489 | 5.0 | 489 | 5.0 | 0.249 | 5.3 | LOS A | 0.0 | 0.0 | 0.00 | 0.45 | 0.00 | 52.1 |
| 5 | T1 | All MCs | 536 | 12.0 | 536 | 12.0 | 0.603 | 49.5 | LOS D | 15.3 | 118.3 | 0.94 | 0.80 | 0.94 | 17.7 |
| 6b | R3 | All MCs | 21 | 12.0 | 21 | 12.0 | 0.305 | 81.5 | LOS F | 1.4 | 10.9 | 1.00 | 0.71 | 1.00 | 20.4 |
| Appro | bach | | 1046 | 8.7 | 1046 | 8.7 | 0.603 | 29.5 | LOS C | 15.3 | 118.3 | 0.50 | 0.63 | 0.50 | 29.9 |
| North | East: [| Dalrymple | Road | (N) | | | | | | | | | | | |
| 24b | L3 | All MCs | 91 | 6.0 | 91 | 6.0 | 0.136 | 12.8 | LOS B | 1.4 | 10.6 | 0.61 | 1.23 | 0.61 | 16.7 |
| 25 | T1 | All MCs | 666 | 6.0 | 666 | 6.0 | *0.839 | 62.0 | LOS E | 22.6 | 166.3 | 1.00 | 0.97 | 1.14 | 30.4 |
| 26a | R1 | All MCs | 160 | 6.0 | 160 | 6.0 | *0.879 | 82.1 | LOS F | 11.4 | 84.1 | 1.00 | 1.00 | 1.31 | 21.0 |
| Appro | bach | | 917 | 6.0 | 917 | 6.0 | 0.879 | 60.6 | LOS E | 22.6 | 166.3 | 0.96 | 1.00 | 1.12 | 26.6 |
| West: | Bays | water Roa | ad (W) | | | | | | | | | | | | |
| 10a | L1 | All MCs | 103 | 10.0 | 103 | 10.0 | 0.054 | 24.4 | LOS C | 0.0 | 0.0 | 0.00 | 0.45 | 0.00 | 52.2 |
| 11 | T1 | All MCs | 718 | 11.0 | 718 | 11.0 | *0.827 | 75.5 | LOS E | 24.1 | 184.8 | 1.00 | 0.96 | 1.11 | 15.7 |
| 12b | R3 | All MCs | 46 | 11.0 | 46 | 11.0 | *0.667 | 101.1 | LOS F | 3.2 | 24.7 | 1.00 | 0.81 | 1.15 | 20.3 |
| Appro | bach | | 867 | 10.9 | 867 | 10.9 | 0.827 | 70.8 | LOS E | 24.1 | 184.8 | 0.88 | 0.89 | 0.98 | 15.0 |
| South | West: | Dalrympl | e Road | (S) | | | | | | | | | | | |
| 30b | L3 | All MCs | 1 | 0.0 | 1 | 0.0 | 0.001 | 23.5 | LOS C | 0.0 | 0.1 | 0.27 | 1.16 | 0.27 | 27.5 |
| 31 | T1 | All MCs | 1301 | 5.0 | 1301 | 5.0 | 0.831 | 48.4 | LOS D | 39.5 | 288.3 | 0.96 | 0.90 | 1.00 | 36.8 |
| 32a | R1 | All MCs | 433 | 5.0 | 433 | 5.0 | *0.877 | 43.5 | LOS D | 18.6 | 135.4 | 1.00 | 1.09 | 1.16 | 21.1 |
| Appro | bach | | 1735 | 5.0 | 1735 | 5.0 | 0.877 | 47.2 | LOS D | 39.5 | 288.3 | 0.97 | 0.95 | 1.04 | 29.9 |
| All Ve | hicles | | 4565 | 7.2 | 4565 | 7.2 | 0.879 | 50.3 | LOS D | 39.5 | 288.3 | 0.84 | 0.88 | 0.92 | 26.4 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

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Site: [10] 2025 BG PM - Dalrymple / Bayswater (General)

Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 104.0 seconds (Site User-Given Phase Times) Site Scenario: 1 | Local Volumes

| Vehi | cle Mo | ovemen | t Perfo | rma | nce | | | | | | | | | | |
|--------|---------|-----------|---------|------|---------|-------|----------------|-------|----------|-------|--------|-------|---------|--------|-------|
| Mov | Turn | Mov | Dem | nand | Ar | rival | Deg. | Aver. | Level of | 95% B | ack Of | Prop. | Eff. 1 | Number | Aver. |
| ID | | Class | F | lows | FI | ows | Satn | Delay | Service | Qu | eue | Qued | Stop of | Cycles | Speed |
| | | | [Total | HV] | [Total | HV] | | | | [Veh. | Dist] | | Rate to | Depart | |
| | | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | | km/h |
| East: | Baysv | vater Roa | id (E) | | | | | | | | | | | | |
| 4a | L1 | All MCs | 552 | 5.0 | 552 | 5.0 | 0.281 | 6.5 | LOS A | 0.0 | 0.0 | 0.00 | 0.45 | 0.00 | 52.1 |
| 5 | T1 | All MCs | 349 | 12.0 | 349 | 12.0 | *0.829 | 55.7 | LOS E | 9.7 | 74.8 | 1.00 | 0.97 | 1.26 | 15.8 |
| 6b | R3 | All MCs | 21 | 12.0 | 21 | 12.0 | 0.121 | 53.0 | LOS D | 1.0 | 7.7 | 0.93 | 0.71 | 0.93 | 25.6 |
| Appro | bach | | 922 | 7.8 | 922 | 7.8 | 0.829 | 26.2 | LOS C | 9.7 | 74.8 | 0.40 | 0.65 | 0.50 | 33.2 |
| North | East: I | Dalrymple | e Road | (N) | | | | | | | | | | | |
| 24b | L3 | All MCs | 147 | 6.0 | 147 | 6.0 | 0.157 | 11.7 | LOS B | 1.7 | 12.2 | 0.47 | 1.31 | 0.47 | 21.1 |
| 25 | T1 | All MCs | 1046 | 6.0 | 1046 | 6.0 | *0.850 | 45.9 | LOS D | 27.7 | 204.2 | 1.00 | 0.98 | 1.13 | 35.8 |
| 26a | R1 | All MCs | 94 | 6.0 | 94 | 6.0 | 0.408 | 58.5 | LOS E | 4.5 | 33.4 | 0.97 | 0.77 | 0.97 | 27.0 |
| Appro | bach | | 1287 | 6.0 | 1287 | 6.0 | 0.850 | 42.9 | LOS D | 27.7 | 204.2 | 0.93 | 1.01 | 1.04 | 32.0 |
| West: | Bays | water Roa | ad (W) | | | | | | | | | | | | |
| 10a | L1 | All MCs | 136 | 10.0 | 136 | 10.0 | 0.072 | 7.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.45 | 0.00 | 52.2 |
| 11 | T1 | All MCs | 568 | 11.0 | 568 | 11.0 | * 0.843 | 54.9 | LOS D | 16.4 | 125.4 | 1.00 | 1.00 | 1.21 | 16.7 |
| 12b | R3 | All MCs | 175 | 11.0 | 175 | 11.0 | 0.599 | 54.2 | LOS D | 8.4 | 64.0 | 0.96 | 0.82 | 0.96 | 26.9 |
| Appro | bach | | 879 | 10.8 | 879 | 10.8 | 0.843 | 47.4 | LOS D | 16.4 | 125.4 | 0.84 | 0.88 | 0.98 | 21.9 |
| South | West: | Dalrymp | le Road | (S) | | | | | | | | | | | |
| 30b | L3 | All MCs | 1 | 0.0 | 1 | 0.0 | 0.001 | 7.2 | LOS A | 0.0 | 0.0 | 0.14 | 1.00 | 0.14 | 31.6 |
| 31 | T1 | All MCs | 561 | 5.0 | 561 | 5.0 | 0.437 | 28.9 | LOS C | 11.1 | 80.9 | 0.83 | 0.70 | 0.83 | 40.8 |
| 32a | R1 | All MCs | 203 | 5.0 | 203 | 5.0 | *0.880 | 62.4 | LOS E | 11.7 | 85.8 | 1.00 | 1.03 | 1.34 | 23.6 |
| Appro | bach | | 765 | 5.0 | 765 | 5.0 | 0.880 | 37.8 | LOS D | 11.7 | 85.8 | 0.87 | 0.79 | 0.96 | 35.7 |
| All Ve | hicles | | 3854 | 7.3 | 3854 | 7.3 | 0.880 | 38.9 | LOS D | 27.7 | 204.2 | 0.77 | 0.85 | 0.88 | 31.0 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

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Site: [11] 2025 BG PM + DEV - Dalrymple / Bayswater (General)

Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 105.0 seconds (Site User-Given Phase Times) Site Scenario: 1 | Local Volumes

| Vehi | cle Mo | ovemen | t Perfo | rma | nce | | | | | | | | | | |
|--------|---------|-----------|---------|-------|---------|-------|--------|-------|----------|-------|--------|-------|---------|--------|-------|
| Mov | Turn | Mov | Den | nand | Ar | rival | Deg. | Aver. | Level of | 95% B | ack Of | Prop. | Eff. I | Number | Aver. |
| ID | | Class | F | lows | F | lows | Satn | Delay | Service | Qu | eue | Qued | Stop of | Cycles | Speed |
| | | | [Total | HV] | [Total | HV] | | | | [Veh. | Dist] | | Rate to | Depart | |
| | | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | | km/h |
| East: | Baysv | vater Roa | id (E) | | | | | | | | | | | | |
| 4a | L1 | All MCs | 552 | 5.0 | 552 | 5.0 | 0.281 | 6.5 | LOS A | 0.0 | 0.0 | 0.00 | 0.45 | 0.00 | 52.1 |
| 5 | T1 | All MCs | 353 | 12.0 | 353 | 12.0 | *0.844 | 57.2 | LOS E | 10.0 | 77.1 | 1.00 | 0.98 | 1.28 | 15.5 |
| 6b | R3 | All MCs | 21 | 12.0 | 21 | 12.0 | 0.122 | 53.6 | LOS D | 1.0 | 7.8 | 0.93 | 0.71 | 0.93 | 25.4 |
| Appro | bach | | 925 | 7.8 | 925 | 7.8 | 0.844 | 26.9 | LOS C | 10.0 | 77.1 | 0.40 | 0.66 | 0.51 | 32.8 |
| North | East: [| Dalrymple | e Road | (N) | | | | | | | | | | | |
| 24b | L3 | All MCs | 147 | 6.0 | 147 | 6.0 | 0.159 | 12.2 | LOS B | 1.7 | 12.8 | 0.48 | 1.31 | 0.48 | 21.0 |
| 25 | T1 | All MCs | 1046 | 6.0 | 1046 | 6.0 | *0.863 | 48.5 | LOS D | 28.6 | 210.2 | 1.00 | 1.01 | 1.15 | 35.1 |
| 26a | R1 | All MCs | 94 | 6.0 | 94 | 6.0 | 0.412 | 59.7 | LOS E | 4.6 | 33.8 | 0.97 | 0.77 | 0.97 | 26.9 |
| Appro | bach | | 1287 | 6.0 | 1287 | 6.0 | 0.863 | 45.1 | LOS D | 28.6 | 210.2 | 0.94 | 1.02 | 1.06 | 31.3 |
| West | Bays | water Roa | ad (W) | | | | | | | | | | | | |
| 10a | L1 | All MCs | 136 | 10.0 | 136 | 10.0 | 0.072 | 8.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.45 | 0.00 | 52.2 |
| 11 | T1 | All MCs | 581 | 11.0 | 581 | 11.0 | *0.837 | 55.2 | LOS E | 16.9 | 129.4 | 1.00 | 0.99 | 1.20 | 16.9 |
| 12b | R3 | All MCs | 195 | 11.0 | 195 | 11.0 | 0.642 | 55.1 | LOS E | 9.5 | 72.7 | 0.97 | 0.83 | 0.99 | 26.8 |
| Appro | bach | | 912 | 10.9 | 912 | 10.9 | 0.837 | 48.3 | LOS D | 16.9 | 129.4 | 0.84 | 0.87 | 0.97 | 21.7 |
| South | West: | Dalrymp | le Road | I (S) | | | | | | | | | | | |
| 30b | L3 | All MCs | 1 | 0.0 | 1 | 0.0 | 0.001 | 7.2 | LOS A | 0.0 | 0.0 | 0.14 | 1.00 | 0.14 | 31.6 |
| 31 | T1 | All MCs | 561 | 5.0 | 561 | 5.0 | 0.441 | 29.5 | LOS C | 11.2 | 82.0 | 0.83 | 0.71 | 0.83 | 40.5 |
| 32a | R1 | All MCs | 203 | 5.0 | 203 | 5.0 | *0.888 | 63.9 | LOS E | 12.0 | 87.3 | 1.00 | 1.04 | 1.36 | 23.2 |
| Appro | bach | | 765 | 5.0 | 765 | 5.0 | 0.888 | 38.6 | LOS D | 12.0 | 87.3 | 0.88 | 0.80 | 0.97 | 35.4 |
| All Ve | hicles | | 3889 | 7.4 | 3889 | 7.4 | 0.888 | 40.2 | LOS D | 28.6 | 210.2 | 0.78 | 0.86 | 0.89 | 30.4 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

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Site: [12] 2025 BG AM + DEV - Dalrymple / Bayswater (General)

Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 133.0 seconds (Site User-Given Phase Times) Site Scenario: 1 | Local Volumes

| Vehi | cle Mo | ovement | Perfo | rma | nce | | | | | | | | | | |
|--------|---------|-----------|---------|------|---------|-------|--------|-------|----------|-------|--------|-------|---------|--------|-------|
| Mov | Turn | Mov | Dem | nand | Ar | rival | Deg. | Aver. | Level of | 95% B | ack Of | Prop. | Eff. 1 | Number | Aver. |
| ID | | Class | F | lows | F | lows | Satn | Delay | Service | Qu | eue | Qued | Stop of | Cycles | Speed |
| | | | [Total | HV] | [Total | HV] | | | | [Veh. | Dist] | | Rate to | Depart | |
| | | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | | km/h |
| East: | Baysv | vater Roa | d (E) | | | | | | | | | | | | |
| 4a | L1 | All MCs | 489 | 5.0 | 489 | 5.0 | 0.249 | 5.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.45 | 0.00 | 52.1 |
| 5 | T1 | All MCs | 548 | 12.0 | 548 | 12.0 | 0.608 | 50.7 | LOS D | 15.9 | 122.8 | 0.94 | 0.80 | 0.94 | 17.6 |
| 6b | R3 | All MCs | 21 | 12.0 | 21 | 12.0 | 0.310 | 84.2 | LOS F | 1.4 | 11.1 | 1.00 | 0.71 | 1.00 | 20.2 |
| Appro | bach | | 1059 | 8.8 | 1059 | 8.8 | 0.608 | 30.4 | LOS C | 15.9 | 122.8 | 0.51 | 0.64 | 0.51 | 29.4 |
| North | East: I | Dalrymple | Road | (N) | | | | | | | | | | | |
| 24b | L3 | All MCs | 91 | 6.0 | 91 | 6.0 | 0.135 | 13.0 | LOS B | 1.5 | 10.9 | 0.61 | 1.23 | 0.61 | 16.6 |
| 25 | T1 | All MCs | 698 | 6.0 | 698 | 6.0 | *0.869 | 66.8 | LOS E | 25.0 | 184.0 | 1.00 | 1.01 | 1.18 | 29.5 |
| 26a | R1 | All MCs | 160 | 6.0 | 160 | 6.0 | *0.892 | 87.2 | LOS F | 11.7 | 86.3 | 1.00 | 1.02 | 1.34 | 20.5 |
| Appro | bach | | 948 | 6.0 | 948 | 6.0 | 0.892 | 65.1 | LOS E | 25.0 | 184.0 | 0.96 | 1.03 | 1.15 | 25.8 |
| West | Bays | water Roa | ad (W) | | | | | | | | | | | | |
| 10a | L1 | All MCs | 103 | 10.0 | 103 | 10.0 | 0.054 | 25.1 | LOS C | 0.0 | 0.0 | 0.00 | 0.45 | 0.00 | 52.2 |
| 11 | T1 | All MCs | 721 | 11.0 | 721 | 11.0 | *0.821 | 76.0 | LOS E | 24.5 | 187.5 | 1.00 | 0.95 | 1.10 | 15.7 |
| 12b | R3 | All MCs | 52 | 11.0 | 52 | 11.0 | *0.754 | 104.3 | LOS F | 3.7 | 28.4 | 1.00 | 0.86 | 1.25 | 19.9 |
| Appro | bach | | 876 | 10.9 | 876 | 10.9 | 0.821 | 71.7 | LOS E | 24.5 | 187.5 | 0.88 | 0.88 | 0.98 | 14.9 |
| South | West: | Dalrympl | e Road | (S) | | | | | | | | | | | |
| 30b | L3 | All MCs | 1 | 0.0 | 1 | 0.0 | 0.001 | 24.1 | LOS C | 0.0 | 0.1 | 0.27 | 1.17 | 0.27 | 27.4 |
| 31 | T1 | All MCs | 1301 | 5.0 | 1301 | 5.0 | 0.830 | 49.1 | LOS D | 40.0 | 292.1 | 0.96 | 0.89 | 0.99 | 36.7 |
| 32a | R1 | All MCs | 433 | 5.0 | 433 | 5.0 | *0.890 | 46.3 | LOS D | 19.3 | 141.0 | 1.00 | 1.11 | 1.18 | 20.7 |
| Appro | bach | | 1735 | 5.0 | 1735 | 5.0 | 0.890 | 48.4 | LOS D | 40.0 | 292.1 | 0.97 | 0.95 | 1.04 | 29.6 |
| All Ve | hicles | | 4618 | 7.2 | 4618 | 7.2 | 0.892 | 52.1 | LOS D | 40.0 | 292.1 | 0.85 | 0.88 | 0.93 | 25.9 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

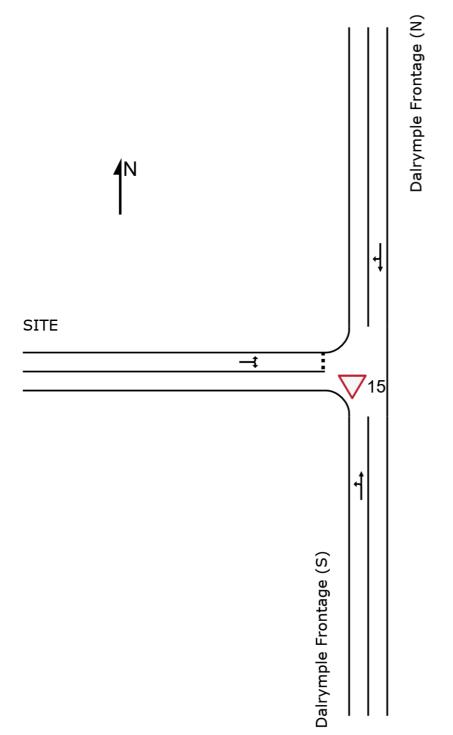
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SITE LAYOUT V Site: [15] 2025 BG AM - Dalrymple Frontage / Site (General)

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



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Site: [15] 2025 BG AM - Dalrymple Frontage / Site (General) Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

| Vehi | cle Mo | ovemen | t Performa | nce | | | | | | | | | |
|--------|----------|----------|--------------|--------------|-------|-------|----------|-------|--------|-------|---------|----------|-------|
| Mov | Turn | Mov | Demand | Arrival | Deg. | Aver. | Level of | 95% E | ack Of | Prop. | Eff. | Number | Aver. |
| ID | | Class | Flows | Flows | Satn | Delay | Service | Qu | eue | Qued | Stop o | f Cycles | Speed |
| | | | [Total HV] | [Total HV] | | | | [Veh. | Dist] | | Rate to | o Depart | |
| | | | veh/h % | veh/h % | v/c | sec | | veh | m | | | | km/h |
| South | n: Dalry | mple Fro | ontage (S) | | | | | | | | | | |
| 1 | L2 | All MCs | 15 0.0 | 15 0.0 | 0.010 | 3.9 | LOS A | 0.0 | 0.0 | 0.00 | 0.44 | 0.00 | 51.3 |
| 2 | T1 | All MCs | 4 12.0 | 4 12.0 | 0.010 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.44 | 0.00 | 54.9 |
| Appro | bach | | 19 2.7 | 19 2.7 | 0.010 | 3.0 | NA | 0.0 | 0.0 | 0.00 | 0.44 | 0.00 | 52.0 |
| North | : Dalry | mple Fro | ontage (N) | | | | | | | | | | |
| 8 | T1 | All MCs | 8 12.0 | 8 12.0 | 0.008 | 0.0 | LOS A | 0.0 | 0.2 | 0.05 | 0.23 | 0.05 | 55.9 |
| 9 | R2 | All MCs | 5 0.0 | 5 0.0 | 0.008 | 5.5 | LOS A | 0.0 | 0.2 | 0.05 | 0.23 | 0.05 | 55.0 |
| Appro | bach | | 14 7.4 | 14 7.4 | 0.008 | 2.1 | NA | 0.0 | 0.2 | 0.05 | 0.23 | 0.05 | 55.4 |
| West | SITE | | | | | | | | | | | | |
| 10 | L2 | All MCs | 11 0.0 | 11 0.0 | 0.014 | 5.5 | LOS A | 0.0 | 0.3 | 0.04 | 0.57 | 0.04 | 52.8 |
| 12 | R2 | All MCs | 9 0.0 | 9 0.0 | 0.014 | 5.5 | LOS A | 0.0 | 0.3 | 0.04 | 0.57 | 0.04 | 48.1 |
| Appro | bach | | 20 0.0 | 20 0.0 | 0.014 | 5.5 | LOS A | 0.0 | 0.3 | 0.04 | 0.57 | 0.04 | 51.2 |
| All Ve | hicles | | 53 2.9 | 53 2.9 | 0.014 | 3.7 | NA | 0.0 | 0.3 | 0.03 | 0.43 | 0.03 | 52.5 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: [16] 2025 BG PM - Dalrymple Frontage / Site (General) Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

| Vehi | cle Mo | ovemen | t Performa | nce | | | | | | | | | |
|--------|----------|-----------|--------------|--------------|-------|-------|----------|-------|--------|-------|---------|----------|-------|
| Mov | Turn | Mov | Demand | Arrival | Deg. | Aver. | Level of | 95% B | ack Of | Prop. | Eff. | Number | Aver. |
| ID | | Class | Flows | Flows | Satn | Delay | Service | Qu | eue | Qued | Stop o | f Cycles | Speed |
| | | | [Total HV] | [Total HV] | | | | [Veh. | Dist] | | Rate to | o Depart | |
| | | | veh/h % | veh/h % | v/c | sec | | veh | m | | | | km/h |
| South | n: Dalry | ymple Fro | ontage (S) | | | | | | | | | | |
| 1 | L2 | All MCs | 15 0.0 | 15 0.0 | 0.009 | 3.9 | LOS A | 0.0 | 0.0 | 0.00 | 0.52 | 0.00 | 50.6 |
| 2 | T1 | All MCs | 1 12.0 | 1 12.0 | 0.009 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.52 | 0.00 | 54.1 |
| Appro | bach | | 16 0.8 | 16 0.8 | 0.009 | 3.7 | NA | 0.0 | 0.0 | 0.00 | 0.52 | 0.00 | 50.8 |
| North | : Dalry | mple Fro | ontage (N) | | | | | | | | | | |
| 8 | T1 | All MCs | 63 12.0 | 63 12.0 | 0.038 | 0.0 | LOS A | 0.0 | 0.2 | 0.01 | 0.05 | 0.01 | 59.1 |
| 9 | R2 | All MCs | 5 0.0 | 5 0.0 | 0.038 | 5.5 | LOS A | 0.0 | 0.2 | 0.01 | 0.05 | 0.01 | 56.7 |
| Appro | bach | | 68 11.1 | 68 11.1 | 0.038 | 0.4 | NA | 0.0 | 0.2 | 0.01 | 0.05 | 0.01 | 58.8 |
| West | SITE | | | | | | | | | | | | |
| 10 | L2 | All MCs | 11 0.0 | 11 0.0 | 0.014 | 5.5 | LOS A | 0.1 | 0.4 | 0.02 | 0.58 | 0.02 | 52.9 |
| 12 | R2 | All MCs | 9 0.0 | 9 0.0 | 0.014 | 5.5 | LOS A | 0.1 | 0.4 | 0.02 | 0.58 | 0.02 | 48.2 |
| Appro | bach | | 20 0.0 | 20 0.0 | 0.014 | 5.5 | LOS A | 0.1 | 0.4 | 0.02 | 0.58 | 0.02 | 51.2 |
| All Ve | hicles | | 104 7.4 | 104 7.4 | 0.038 | 1.9 | NA | 0.1 | 0.4 | 0.01 | 0.22 | 0.01 | 55.6 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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abla Site: [17] 2025 BG AM + DEV - Dalrymple Frontage / Site (General)

Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

| Vehic | cle Mo | ovemen | t Performa | nce | | | | | | | | | | |
|-----------|---------|--------------|-----------------|-----|---------------|--------------|----------------|---------------------|---------------|-----------------|---------------|------|---------------------|----------------|
| Mov ID | Turn | Mov Class | Demand Flows | | rival lows | Deg. Satn | Aver. Delav | Level of Service | | Back Of Ieue | Prop. Qued | | Number of Cycles | Aver. Speed |
| | | 01000 | [Total HV] | | | v/c | sec | | [Veh. veh | Dist] m | Quou | | o Depart | km/h |
| South | : Dalry | mple Fro | ontage (S) | | | | | | | | | | | |
| 1 | L2 | All MCs | 63 0.0 | 63 | 0.0 | 0.036 | 3.9 | LOS A | 0.0 | 0.0 | 0.00 | 0.52 | 0.00 | 50.5 |
| 2 | T1 | All MCs | 4 12.0 | 4 | 12.0 | 0.036 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.52 | 0.00 | 54.1 |
| Appro | ach | | 67 0.7 | 67 | 0.7 | 0.036 | 3.7 | NA | 0.0 | 0.0 | 0.00 | 0.52 | 0.00 | 50.7 |
| North: | Dalry | mple Fro | ntage (N) | | | | | | | | | | | |
| 8 | T1 | All MCs | 8 12.0 | 8 | 12.0 | 0.018 | 0.2 | LOS A | 0.1 | 0.6 | 0.16 | 0.42 | 0.16 | 52.6 |
| 9 | R2 | All MCs | 23 0.0 | 23 | 0.0 | 0.018 | 5.6 | LOS A | 0.1 | 0.6 | 0.16 | 0.42 | 0.16 | 53.2 |
| Appro | ach | | 32 3.2 | 32 | 3.2 | 0.018 | 4.2 | NA | 0.1 | 0.6 | 0.16 | 0.42 | 0.16 | 53.1 |
| West: | SITE | | | | | | | | | | | | | |
| 10 | L2 | All MCs | 19 0.0 | 19 | 0.0 | 0.027 | 5.5 | LOS A | 0.1 | 0.7 | 0.04 | 0.57 | 0.04 | 52.8 |
| 12 | R2 | All MCs | 19 0.0 | 19 | 0.0 | 0.027 | 5.5 | LOS A | 0.1 | 0.7 | 0.04 | 0.57 | 0.04 | 48.1 |
| Appro | ach | | 38 0.0 | 38 | 0.0 | 0.027 | 5.5 | LOS A | 0.1 | 0.7 | 0.04 | 0.57 | 0.04 | 51.0 |
| All Ve | hicles | | 137 1.1 | 137 | 1.1 | 0.036 | 4.3 | NA | 0.1 | 0.7 | 0.05 | 0.51 | 0.05 | 51.5 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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abla Site: [18] 2025 BG PM + DEV - Dalrymple Frontage / Site (General)

Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

| Vehic | cle Mo | ovemen | t Performa | nce | | | | | | | | | |
|-----------|---------|--------------|-----------------|-------------------------|--------------|----------------|---------------------|---------------|---------------|---------------|------|--------------------|----------------|
| Mov ID | Turn | Mov Class | Demand Flows | Arrival Flows | Deg. Satn | Aver. Delav | Level of Service | | ack Of eue | Prop. Qued | | Number f Cycles | Aver. Speed |
| | | 01000 | [Total HV] | [Total HV] veh/h % | v/c | sec | | [Veh. veh | Dist] m | Quou | | Depart | km/h |
| South | : Dalry | mple Fro | ontage (S) | | | | | | | | | | |
| 1 | L2 | All MCs | 27 0.0 | 27 0.0 | 0.015 | 3.9 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 50.4 |
| 2 | T1 | All MCs | 1 12.0 | 1 12.0 | 0.015 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 54.0 |
| Appro | ach | | 28 0.4 | 28 0.4 | 0.015 | 3.8 | NA | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 50.6 |
| North | Dalry | mple Fro | ntage (N) | | | | | | | | | | |
| 8 | T1 | All MCs | 63 12.0 | 63 12.0 | 0.041 | 0.0 | LOS A | 0.1 | 0.5 | 0.03 | 0.09 | 0.03 | 58.3 |
| 9 | R2 | All MCs | 11 0.0 | 11 0.0 | 0.041 | 5.5 | LOS A | 0.1 | 0.5 | 0.03 | 0.09 | 0.03 | 56.3 |
| Appro | ach | | 74 10.3 | 74 10.3 | 0.041 | 0.8 | NA | 0.1 | 0.5 | 0.03 | 0.09 | 0.03 | 57.8 |
| West: | SITE | | | | | | | | | | | | |
| 10 | L2 | All MCs | 43 0.0 | 43 0.0 | 0.063 | 5.5 | LOS A | 0.2 | 1.6 | 0.02 | 0.58 | 0.02 | 52.9 |
| 12 | R2 | All MCs | 43 0.0 | 43 0.0 | 0.063 | 5.5 | LOS A | 0.2 | 1.6 | 0.02 | 0.58 | 0.02 | 48.2 |
| Appro | ach | | 86 0.0 | 86 0.0 | 0.063 | 5.5 | LOS A | 0.2 | 1.6 | 0.02 | 0.58 | 0.02 | 51.1 |
| All Ve | hicles | | 188 4.1 | 188 4.1 | 0.063 | 3.4 | NA | 0.2 | 1.6 | 0.02 | 0.38 | 0.02 | 53.2 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: [19] 2035 BG AM - Dalrymple Frontage / Site (General) Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

| Vehi | cle Mo | ovemen | t Performa | nce | | | | | | | | | |
|--------|----------|----------|--------------|--------------|---------|-------|----------|-------|---------|-------|---------|-----------|-------|
| Mov | Turn | Mov | Demand | Arriva | l Deg. | Aver. | Level of | 95% E | Back Of | Prop. | Eff. | Number | Aver. |
| ID | | Class | Flows | Flows | s Satn | Delay | Service | Qu | ieue | Qued | Stop o | of Cycles | Speed |
| | | | [Total HV] | [Total HV] |] | | | [Veh. | Dist] | | Rate to | o Depart | |
| | | | veh/h % | veh/h % | o v/c | sec | | veh | m | | | | km/h |
| South | n: Dalry | mple Fro | ontage (S) | | | | | | | | | | |
| 1 | L2 | All MCs | 15 0.0 | 15 0.0 | 0.011 | 3.9 | LOS A | 0.0 | 0.0 | 0.00 | 0.42 | 0.00 | 51.5 |
| 2 | T1 | All MCs | 5 12.0 | 5 12.0 | 0.011 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.42 | 0.00 | 55.1 |
| Appro | bach | | 20 3.2 | 20 3.2 | 2 0.011 | 2.9 | NA | 0.0 | 0.0 | 0.00 | 0.42 | 0.00 | 52.4 |
| North | : Dalry | mple Fro | ontage (N) | | | | | | | | | | |
| 8 | T1 | All MCs | 11 12.0 | 11 12.0 | 0.009 | 0.0 | LOS A | 0.0 | 0.2 | 0.05 | 0.20 | 0.05 | 56.3 |
| 9 | R2 | All MCs | 5 0.0 | 5 0.0 | 0.009 | 5.5 | LOS A | 0.0 | 0.2 | 0.05 | 0.20 | 0.05 | 55.3 |
| Appro | bach | | 16 8.0 | 16 8.0 | 0.009 | 1.8 | NA | 0.0 | 0.2 | 0.05 | 0.20 | 0.05 | 55.8 |
| West | SITE | | | | | | | | | | | | |
| 10 | L2 | All MCs | 11 0.0 | 11 0.0 | 0.014 | 5.5 | LOS A | 0.0 | 0.3 | 0.04 | 0.57 | 0.04 | 52.8 |
| 12 | R2 | All MCs | 9 0.0 | 9 0.0 | 0.014 | 5.5 | LOS A | 0.0 | 0.3 | 0.04 | 0.57 | 0.04 | 48.1 |
| Appro | bach | | 20 0.0 | 20 0.0 | 0.014 | 5.5 | LOS A | 0.0 | 0.3 | 0.04 | 0.57 | 0.04 | 51.1 |
| All Ve | hicles | | 56 3.4 | 56 3.4 | 0.014 | 3.5 | NA | 0.0 | 0.3 | 0.03 | 0.41 | 0.03 | 52.8 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: [20] 2035 BG PM - Dalrymple Frontage / Site (General) Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

| Vehi | cle Mo | ovemen | t Performa | nce | | | | | | | | | |
|--------|----------|-----------|--------------|--------------|-------|-------|----------|-------------|--------|-------|---------|----------|-------|
| Mov | Turn | Mov | Demand | Arrival | Deg. | Aver. | Level of | 95% B | ack Of | Prop. | Eff. | Number | Aver. |
| ID | | Class | Flows | Flows | Satn | Delay | Service | Qu | eue | Qued | Stop o | f Cycles | Speed |
| | | | [Total HV] | [Total HV] | | | | [Veh. Dist] | | | Rate to | o Depart | |
| | | | veh/h % | veh/h % | v/c | sec | | veh | m | | | | km/h |
| South | n: Dalry | ymple Fro | ontage (S) | | | | | | | | | | |
| 1 | L2 | All MCs | 15 0.0 | 15 0.0 | 0.009 | 3.9 | LOS A | 0.0 | 0.0 | 0.00 | 0.52 | 0.00 | 50.6 |
| 2 | T1 | All MCs | 1 12.0 | 1 12.0 | 0.009 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.52 | 0.00 | 54.1 |
| Appro | bach | | 16 0.8 | 16 0.8 | 0.009 | 3.7 | NA | 0.0 | 0.0 | 0.00 | 0.52 | 0.00 | 50.8 |
| North | : Dalry | mple Fro | ontage (N) | | | | | | | | | | |
| 8 | T1 | All MCs | 77 12.0 | 77 12.0 | 0.045 | 0.0 | LOS A | 0.0 | 0.2 | 0.01 | 0.04 | 0.01 | 59.2 |
| 9 | R2 | All MCs | 5 0.0 | 5 0.0 | 0.045 | 5.5 | LOS A | 0.0 | 0.2 | 0.01 | 0.04 | 0.01 | 56.8 |
| Appro | bach | | 82 11.2 | 82 11.2 | 0.045 | 0.3 | NA | 0.0 | 0.2 | 0.01 | 0.04 | 0.01 | 59.0 |
| West | SITE | | | | | | | | | | | | |
| 10 | L2 | All MCs | 11 0.0 | 11 0.0 | 0.014 | 5.5 | LOS A | 0.1 | 0.4 | 0.02 | 0.58 | 0.02 | 52.9 |
| 12 | R2 | All MCs | 9 0.0 | 9 0.0 | 0.014 | 5.5 | LOS A | 0.1 | 0.4 | 0.02 | 0.58 | 0.02 | 48.2 |
| Appro | bach | | 20 0.0 | 20 0.0 | 0.014 | 5.5 | LOS A | 0.1 | 0.4 | 0.02 | 0.58 | 0.02 | 51.2 |
| All Ve | hicles | | 118 7.9 | 118 7.9 | 0.045 | 1.7 | NA | 0.1 | 0.4 | 0.01 | 0.19 | 0.01 | 56.0 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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abla Site: [21] 2035 BG AM + DEV - Dalrymple Frontage / Site (General)

Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

| Vehicle Movement Performance | | | | | | | | | | | | | | | |
|------------------------------|-------------------------------|---------|-----------|------|---------|--------|-------|-------|----------|-------|---------|-------|---------|----------|-------|
| Mov | Turn | Mov | Dema | and | Ar | rrival | Deg. | Aver. | Level of | 95% | Back Of | Prop. | Eff. | Number | Aver. |
| ID | | Class | Flo | ows | F | lows | Satn | Delay | Service | Q | ueue | Qued | Stop o | f Cycles | Speed |
| | | | [Total F | HV] | [Total | HV] | | | | [Veh. | Dist] | | Rate to | o Depart | |
| | | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | | km/h |
| South | South: Dalrymple Frontage (S) | | | | | | | | | | | | | | |
| 1 | L2 | All MCs | 63 | 0.0 | 63 | 0.0 | 0.037 | 3.9 | LOS A | 0.0 | 0.0 | 0.00 | 0.51 | 0.00 | 50.6 |
| 2 | T1 | All MCs | 5 1 | 2.0 | 5 | 12.0 | 0.037 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.51 | 0.00 | 54.2 |
| Appro | ach | | 68 | 0.9 | 68 | 0.9 | 0.037 | 3.6 | NA | 0.0 | 0.0 | 0.00 | 0.51 | 0.00 | 50.9 |
| North | North: Dalrymple Frontage | | | | | | | | | | | | | | |
| 8 | T1 | All MCs | 11 1 | 2.0 | 11 | 12.0 | 0.019 | 0.2 | LOS A | 0.1 | 0.6 | 0.16 | 0.40 | 0.16 | 52.9 |
| 9 | R2 | All MCs | 23 | 0.0 | 23 | 0.0 | 0.019 | 5.6 | LOS A | 0.1 | 0.6 | 0.16 | 0.40 | 0.16 | 53.4 |
| Appro | ach | | 34 | 3.8 | 34 | 3.8 | 0.019 | 3.9 | NA | 0.1 | 0.6 | 0.16 | 0.40 | 0.16 | 53.3 |
| West: | SITE | | | | | | | | | | | | | | |
| 10 | L2 | All MCs | 19 | 0.0 | 19 | 0.0 | 0.027 | 5.5 | LOS A | 0.1 | 0.7 | 0.05 | 0.57 | 0.05 | 52.8 |
| 12 | R2 | All MCs | 19 | 0.0 | 19 | 0.0 | 0.027 | 5.5 | LOS A | 0.1 | 0.7 | 0.05 | 0.57 | 0.05 | 48.1 |
| Appro | ach | | 38 | 0.0 | 38 | 0.0 | 0.027 | 5.5 | LOS A | 0.1 | 0.7 | 0.05 | 0.57 | 0.05 | 51.0 |
| All Ve | hicles | | 140 | 1.4 | 140 | 1.4 | 0.037 | 4.2 | NA | 0.1 | 0.7 | 0.05 | 0.50 | 0.05 | 51.6 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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abla Site: [22] 2035 BG PM + DEV - Dalrymple Frontage / Site (General)

Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

| Vehicle Movement Performance | | | | | | | | | | | | | |
|------------------------------|---------|--------------|-----------------|-------------------------|--------------|----------------|---------------------|---------------|---------------|---------------|------|--------------------|----------------|
| Mov ID | Turn | Mov Class | Demand Flows | Arrival Flows | Deg. Satn | Aver. Delav | Level of Service | | ack Of eue | Prop. Qued | | Number f Cycles | Aver. Speed |
| | | | [Total HV] | [Total HV] veh/h % | v/c | sec | | [Veh. veh | Dist] m | | | Depart | ˈ km/h |
| South | : Dalry | /mple Fro | ontage (S) | | | | | | | | | | |
| 1 | L2 | All MCs | 27 0.0 | 27 0.0 | 0.015 | 3.9 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 50.4 |
| 2 | T1 | All MCs | 1 12.0 | 1 12.0 | 0.015 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 54.0 |
| Appro | ach | | 28 0.4 | 28 0.4 | 0.015 | 3.8 | NA | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 50.6 |
| North | Dalry | mple Fro | ntage (N) | | | | | | | | | | |
| 8 | T1 | All MCs | 77 12.0 | 77 12.0 | 0.048 | 0.0 | LOS A | 0.1 | 0.5 | 0.03 | 0.07 | 0.03 | 58.5 |
| 9 | R2 | All MCs | 11 0.0 | 11 0.0 | 0.048 | 5.5 | LOS A | 0.1 | 0.5 | 0.03 | 0.07 | 0.03 | 56.4 |
| Appro | ach | | 87 10.6 | 87 10.6 | 0.048 | 0.7 | NA | 0.1 | 0.5 | 0.03 | 0.07 | 0.03 | 58.1 |
| West: | SITE | | | | | | | | | | | | |
| 10 | L2 | All MCs | 43 0.0 | 43 0.0 | 0.063 | 5.5 | LOS A | 0.2 | 1.6 | 0.02 | 0.58 | 0.02 | 52.9 |
| 12 | R2 | All MCs | 43 0.0 | 43 0.0 | 0.063 | 5.5 | LOS A | 0.2 | 1.6 | 0.02 | 0.58 | 0.02 | 48.2 |
| Appro | ach | | 86 0.0 | 86 0.0 | 0.063 | 5.5 | LOS A | 0.2 | 1.6 | 0.02 | 0.58 | 0.02 | 51.1 |
| All Ve | hicles | | 202 4.6 | 202 4.6 | 0.063 | 3.2 | NA | 0.2 | 1.6 | 0.02 | 0.35 | 0.02 | 53.5 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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